APPENDIX 6 2025 ANNUAL GEOTECHNICAL INSPECTION REPORT

2025 Annual Geotechnical Inspection
Meliadine Gold Mine, Nunavut, Canada

PRESENTED TO

Agnico Eagle Mines Limited

MARCH 4, 2026
ISSUED FOR USE
FILE: 704-ENG.EARC03140-46
AGNICO EAGLE DOCUMENT NUMBER: 65-600-230-TCR-003

Tetra Tech Canada Inc.
14940 - 123 Avenue
Edmonton, AB T5V 1B4 CANADA
Tel 780.451.2121 Fax 780.454.5688

This page intentionally left blank.

EXECUTIVE SUMMARY

Tetra Tech Canada Inc. (Tetra Tech) was retained by Agnico Eagle Mines Limited (Agnico Eagle) to conduct the 2025 annual geotechnical inspection for the Meliadine Gold Mine (the Meliadine Mine), located approximately 25 km north of Rankin Inlet, in the Kivalliq Region of Nunavut. The Meliadine Mine consists of underground development and open pits for the extraction of gold ore.

The geotechnical inspection is pursuant to the requirements of the amended Type A Water Licence Permit No. 2AM-MEL1631 (Nunavut Water Board 2024). Under Part I, Item 13 (Page 23) and Schedule I, Item 1 (Page 41) of the Water Licence, Agnico Eagle is required to undertake an annual geotechnical inspection of its facilities between July and September each year. The inspection took place from August 28, 2025, to September 1, 2025, and was conducted by Hongwei Xia of Tetra Tech, a Geotechnical Engineer, holding professional registration in Nunavut, and Devon Sosniuk of Tetra Tech, a Geotechnical Engineer-in-Training. A summary of the findings was presented to Agnico Eagle at a close out meeting on September 1, 2025.

The inspection included water collection ponds (CP), dikes (D-CP), saline water collection ponds (SP), roads, landfills, landfarms, and other geotechnical structures. The following is a summary of general observations from the site inspection.

CP1 and Dike D-CP1

CP1, Dike D-CP1, downstream seepage collection ditches, sump, and Jetty1 are performing adequately. Some erosion was observed on the upstream shell of Dike D-CP1 and occurred during a high-water event between 2019 and 2020. No noticeable change in the upstream shell erosion has been observed since 2021. Ongoing erosion monitoring should be conducted to determine whether remedial measures are required.

In 2023, ponding water was observed at various locations along the downstream collection ditches. In 2024, the channel was regraded to maintain a positive gradient and promote water flow. Additional rockfill was placed to cover the east shoulder of the collection ditches to reduce the potential for permafrost degradation. In 2025, additional cover was placed over the south end of D-CP1 to facilitate the expansion of the CP1 pipeline crossing.

To help mitigate the warming trend of the dike's foundation, it is recommended that snow removal and snow fencing be continued and improved. It is also recommended to perform a thermal performance review and calibration of D-CP1 and to develop a mitigation plan to stabilize the dike's foundation temperature.

CP2 and its Associated Infrastructure

CP2 and its associated infrastructure (Channel9, Channel9 Berm, Channel10, and CP2 Thermal Berm) are performing adequately. In 2025, rockfill was placed between Channel9, Channel10, and the Waste Rock Storage Facility 3 (WRSF3) toe to mitigate the thaw subsidence and cracking observed in the previous annual inspections. Overall, CP2 and its associated infrastructure are performing well.

CP3 and its Associated Infrastructure

CP3 and its associated infrastructure (CP3 Thermal Berm, Berm2, and the reconstructed Channel3) are performing adequately.

CP4 and its Associated Infrastructure

CP4 and its associated infrastructure (CP4 Thermal Berm and Channel4) are performing adequately. The original ground along the east and north sides of CP4 and the area between CP4 and WRSF1 were covered with additional rockfill for thermal protection in 2023. The pond slopes appear to be stable.

In 2024, a rockfill berm was constructed on the downstream shoulder of Channel4 to remediate erosion observed during previous annual geotechnical inspections and improve channel capacity. Thaw subsidence, exposed geotextile, cracking, and settlement of overburden material were observed at several locations along the upstream shoulder of Channel4. The cracks and thaw subsidence indicate localized thermal degradation and were likely caused by surface runoff over the area between WRSF1 and Channel4. During the 2025 annual geotechnical inspection, ongoing cover placement was observed between Channel4 and WRSF1 to mitigate the thaw subsidence and cracking as recommended during the 2024 annual geotechnical inspection. Agnico Eagle informed Tetra Tech that the rockfill cover placement between Channel4 and WRSF1 was completed after the 2025 annual geotechnical inspection. Tetra Tech will verify the work during the 2026 annual geotechnical inspection.

Minor ponding of water was observed at various localized areas along Channel4. Given the size of the ponded area, the overall performance of Channel4 is not expected to be compromised.

Overall, Channel4 is performing as designed. No geotechnical concerns were noted on the CP4 Thermal Berm, and the original ground below the CP4 Thermal Berm is in a frozen condition.

CP5 and Dike D-CP5

CP5 and Dike D-CP5 are performing adequately. A water diversion ditch was previously excavated around the jetty to divert water from localized areas towards the jetty. Minor slope erosion was observed at various locations along the ditch due to the excavation. The performance of the jetty is not expected to be impacted by the ditch.

CP6 and its Associated Infrastructure

CP6 and its associated infrastructure (CP6 Thermal Berm) are performing adequately. The rockfill cover placed in previous years between WRSF3 and Pond CP6 to provide thermal and erosion protection appeared to be performing adequately. In 2025, additional rockfill was placed between the east side of Pond CP6, Channel9, and WRSF3 to provide additional thermal and erosion protection. The small ponding area between the CP6 access ramp and CP6 Thermal Berm was filled with coarse rockfill in 2024 to avoid ponding in the area. No ponding water was observed at this location during the 2025 annual geotechnical inspection.

CP9 Water Management Infrastructure (WMI)

In the winter of 2025, CP9 WMI (CP9 Thermal Berm, Berm4, and Channel11) was constructed to support the construction and operation of WRSF6 and mining activities at Pump. As of the 2025 annual geotechnical inspection, construction of CP9 (formed by Pump01 open pit) is ongoing.

CP9 Thermal Berm, Berm4, and Channel11 are performing adequately. Cracking was observed at one location along the crest of the CP9 Thermal Berm, and ponding water was observed along the downstream and upstream toes of the berm. The cracking and ponding water on the downstream side of the berm should be monitored and are not expected to affect the berm’s performance. The ponding water on the upstream side of the CP9 Thermal Berm should be pumped, or the area should be filled and graded to promote drainage. Ponding water was also observed between WRSF6 and the upstream toe of Berm4. It is recommended to fill and grade depressions between WRSF6 and Berm4 to facilitate drainage.

Saline Ponds

SP1 and SP3 are performing adequately. The cracks on the thermal cover slope observed since 2020 have not changed. No other permafrost degradation was observed, except for the cracks noted here.

Diversion Channels and Berms

The diversion channels and berms are performing well, with some maintenance required. The recommendations are outlined below:

- Tetra Tech was informed by Agnico Eagle that maintenance and repair at Channel7 were fully completed after the 2025 annual geotechnical inspection. Tetra Tech will confirm the maintenance and repairs at Channel7 during the 2026 annual geotechnical inspection.
- Agnico Eagle completed most maintenance and repair at Channel1, except for a small section in the lower reach after the 2025 annual geotechnical inspection, and Tetra Tech will confirm the maintenance work conducted at Channel1 during the 2026 annual geotechnical inspection.
- It is recommended that the eastern portion of Channel5 be repaired and maintained. The western portion of the channel was reconstructed and is functioning well.
- In 2024, minor erosion was observed during the inspection of Berm3, as its cover materials are susceptible to erosion. During the 2025 inspection, the erosion and cracking previously observed at the top of Berm3 had increased. The cracking has widened and deepened. The top of Berm3 should be repaired to remediate cracking. Slope erosion should be monitored, and consideration should be given to placing coarser material on Berm3 to reduce the potential for erosion if it becomes substantial.

Tailings Storage Facility

The Tailings Storage Facility (TSF) appeared to be functioning well at the time of the inspection. Tailings placement appears to be following the construction protocol established in the Operation, Maintenance, and Surveillance (OMS) manual and design report. Ground temperatures are being collected at the specified frequency in the OMS manual.

The TSF perimeter rockfill berm appears to be functioning well from a geotechnical perspective with no signs of distress. Cracking and erosion of the tailings along the toe of the exposed north slope of Cell 1 was observed in 2023. A rockfill berm was constructed in 2023 between the interface of Cell 1 and Cell 2 to reduce the erosion caused by surface runoff. Since its construction, the rockfill berm has been buried from tailings placement in Cell 2. The crest and north slope of Cell 1 were covered with rockfill to prevent surface erosion, and it appears to be functioning adequately. Previously observed minor cracking along the crest of the rockfill berm at Cell 1, was not observed during the 2025 annual geotechnical inspection.

WRSF1

WRSF1 107 m bench had been constructed to the design limits at the time of the 2025 annual geotechnical inspection. A portion of the 112 m bench was filled with waste rock in 2025, and another portion of the 112 m bench was used for the temporary storage of mineralized waste for future processing. WRSF1 was generally constructed according to design and is performing well.

WRSF3

In general, WRSF3 is performing well. Ponded water was observed at the southwest corner of WRSF3 at the time of the inspection. It is understood that Agnico Eagle conducts regular pumping to manage the ponded water. Tetra

Tech recommends that regular pumping continues during operations until alternative water management strategies are implemented to manage the ponding water at the WRSF3 toe.

WRSF6

Material placement on the pile generally follows the WRSF6 design. New waste material appeared to have been placed in accordance with the construction protocol established in the OMS manual. In general, WRSF6 is performing well.

Site Roads

The site's mine roads and culverts were generally well-maintained and in good geotechnical condition at the time of the inspection. No specific recommendations for geotechnical improvements are provided.

Borrow Sources

The borrow sources are well maintained and no geotechnical issues were observed during the time of the inspection. No specific recommendations for geotechnical improvements are provided.

Ore Stockpiles

The ore stockpiles are generally well maintained and performing well with no geotechnical issues during the time of the inspection. No specific recommendations for geotechnical improvements are provided.

Landfill

In general, the landfill is performing well, with no geotechnical issues noted during the inspection.

Industrial Fuel Tank Farm

Cracking observed at the crest of the berms at the Industrial Fuel Tank Farm has increased in length and depth from 2024 to 2025. The repair work for the cracking was completed by Agnico Eagle in December 2025. Tetra Tech will confirm the repairs during the 2026 annual geotechnical inspection.

Portal No. 1 and Portal No. 2

In general, Portal No. 1 and Portal No. 2 are performing adequately.

The footing foundations supporting the corrugated steel entry to Portal No. 2 have eroded over recent years. It is recommended that the voids underneath the footings of Portal No. 2 be backfilled, and erosion protection measures be put in place to prevent additional erosion.

Mine Site Fuel Tank Farm

The Mine Site Fuel Tank Farm is performing well. During the inspection, exposed geotextile and liner were observed on the downstream slope of the southwest perimeter berm. It is recommended that additional liner cover material be placed at the exposed liner area, and the downstream slope is regraded to design.

Pond P3

Pond P3 and its associated structures are performing adequately. Localized ponding was observed in the upstream ditch and is not expected to affect its performance. Cracking and erosion were observed along the crest of

Berm DP3-A. The erosion was most prevalent beneath the water pipelines, which run across DP3-A to SP3. The eroded areas of Berm DP3-A should be regraded, and the water pipelines should be monitored for leaks.

Other Facilities

No other geotechnical issues were noted in other facilities inspected, including the Cyanide Storage Pad, Emulsion Plant Storage, Freshwater Intake, Incinerator Pad, Paste Plant Ramp, and Industrial Pad. No permafrost degradation was observed around these facilities during the inspection.

Exploration Camp and Access Road

Portions of the exploration camp and its associated structures were being dismantled and decommissioned during the annual inspection. The exploration camp and access road are performing well and were in good geotechnical condition at the time of the inspection.

All-weather Access Road

In general, the All-weather Access Road (AWAR) appeared to be in good geotechnical condition at the time of the inspection. Site personnel reported that the road performed well during the 2025 freshet, although ponded water was observed in several locations on the side slope of the road. Additional culverts and raising some sections of the road surface would reduce the risk of the road overtopping during significant freshet events. Construction of the new waterline along the AWAR has blocked several culverts and pipe sleeves, which were used to pass a hose through for freshet pumping activities, resulting in high water levels observed against the road. The new waterline has also increased the potential for erosion and sediment movement due to the use of bedding material along the AWAR toe and drainage issues. Some culverts lack signs for easy identification, some are mis-labeled, and some KM stations along the AWAR appeared to be out of alignment. It is recommended that the signage for culverts and KM stations be reviewed for accuracy and updated where required.

Itivia Bypass Road

The Itivia Bypass Road was in good condition at the time of the site inspection. A low area of the road northwest of Culvert C10 flooded during the 2019 freshet. The area was raised in late 2019, but the road was overtopped again during the 2020 freshet. This section of road performed better during the 2022 through 2025 freshets, but it is recommended that additional culverts or other measures be implemented to prevent this from occurring in the future. Similar to the AWAR, it was noted that some culverts do not have a sign for their identification and some are mis-labeled. It is recommended that culvert signage be reviewed for accuracy and updated where required.

The fuel tank farm liner and berm were raised in 2024 to accommodate two additional fuel tanks. Ponded water was observed at the Itivia fuel farm, and minor cracking was observed along the northeast perimeter berm at the time of the inspection. It is understood that the ponded water is pumped out annually, Tetra Tech recommends that it be pumped out as soon as practical to reduce the risk of erosion, and the northeast perimeter berm be monitored for future erosion and settlement.

TABLE OF CONTENTS

LIST OF TABLES IN TEXT

APPENDIX SECTIONS

FIGURES

APPENDICES

ACRONYMS & ABBREVIATIONS

Acronyms/Abbreviations	Definition
Agnico Eagle	Agnico Eagle Mines Limited
ATV	All-terrain Vehicle
AWAR	All-weather Access Road
CDA	Canadian Dam Association
CP	Collection Pond
EWTP	Effluent Water Treatment Plant
GTC	Ground Temperature Cable
HDPE	High-Density Polyethylene
IDF	Inflow Design Flood
km	Kilometers
masl	Metres Above Sea Level
mbgs	Metres below ground surface
MSE	Mechanically Stabilized Earth
NWB	Nunavut Water Board
OMS	Operation, Maintenance, and Surveillance
PC	Project Certificate
PGA	Peak Ground Acceleration
ppt	Parts Per Thousand
SP	Saline Pond
SWTP	Saline Water Treatment Plant
Tetra Tech	Tetra Tech Canada Inc.
TSF	Tailings Storage Facility
TSS	Total Suspended Solids
WMI	Water Management Infrastructure
WRSF	Waste Rock Storage Facility

CONFIDENTIALITY STATEMENT

1.0 INTRODUCTION

1.1 General

Tetra Tech Canada Inc. (Tetra Tech) was retained by Agnico Eagle Mines Limited (Agnico Eagle) to conduct the 2025 annual geotechnical inspection for the Meliadine Gold Mine (the Meliadine Mine), located approximately 25 km north of Rankin Inlet, in the Kivalliq Region of Nunavut. A general location plan for the Meliadine Mine is shown in Figure 1. The Meliadine Mine involves two open pits at the Tiriganiaq (TIRI) deposits (TIRI#1 and TIRI#2), one underground development at TIRI, and three open pits at the Pump deposits (Pump01, Pump02, and Pump04), to extract gold ore. Mine development is also planned at the Wesmeg, Wesmeg North, FZone, and Discovery deposits. These operations are being undertaken under the Nunavut Impact Review Board Project Certificate (PC) No. 006 and the amended Nunavut Water Board (NWB) Type A Water Licence No. 2AM-MEL1631 (NWB 2024).

The geotechnical inspection was conducted pursuant to the requirements of the Amended Type A Water Licence. Under Part I, Item 13 (Page 23) and Schedule I, Item 1 (Page 41) of the Water Licence, Agnico Eagle is required to undertake an annual geotechnical inspection of its facilities by a Geotechnical Engineer between July and September. The 2025 annual geotechnical inspection took place from August 28, 2025, to September 1, 2025, and was conducted by Hongwei Xia of Tetra Tech, a Geotechnical Engineer, holding professional registration in Nunavut, and Devon Sosniuk of Tetra Tech, a Geotechnical Engineer-in-Training. Justin Bieber and Prempeh Owusu of Agnico Eagle, on-site geotechnical engineers at the Meliadine Mine, and Marielle Limoges of Agnico Eagle, the Engineer of Record for the critical infrastructure at the Meliadine Mine, accompanied and guided the Tetra Tech staff throughout the inspection and provided invaluable information, that aided in determining the performance of the structures. A close out presentation for the 2025 annual geotechnical inspection was provided by Tetra Tech on September 1, 2025.

The following structures were inspected:

Main Site Including:

- Water collection ponds CP1, CP2, CP3, CP4, CP5, and CP6 and their associated dikes (D-CP1 and D-CP5), thermal and water diversion berms, channels, and jetties.
- Waste Rock Storage Facility 1 (WRSF1), WRSF3, and WRSF6.
- Filtered Tailings Storage Facility (TSF).
- Saline Pond1 (SP1) and Saline Pond3 (SP3).

Site Roads:

- Main site pad area roads, including culverts.
- Tiriganiaq Esker access road.
- Wesmeg access road, Wesmeg Borrow, and vent raise.
- Magazine storage access road.
- Main site water intake access road.
- Emulsion plant pad access road.

- Access roads to water CPs.

Pads:

- Main camp pad.
- Industrial pad.
- East ventilation raise pad.
- Cyanide storage pad.
- Explosives (ANFO plant) pad and magazine storage.
- Emulsion plant pad.
- Crusher ramp and associated Mechanically Stabilized Earth (MSE) walls.
- Paste plant ramp.
- Ore and waste rock storage areas.
- Landfarm.
- Operations landfills.
- Underground Portals No. 1 and No. 2.
- Industrial fuel storage and mine site fuel storage.
- Pond P3.

Exploration Camp Site Including:

- Site pad and diffuser access road.
- Genset storage area.
- Freshwater intake.
- Access road.
- Fuel storage.

All-Weather Access Road (AWAR) and Culverts.

Itivia Site:

- Fuel storage.
- Bypass road and culverts.

The facilities at the main mine site and exploration camp areas in 2025 are shown in Figure 2.

The AWAR connecting Rankin Inlet to the Meliadine Mine provides one-way traffic access (with pull-outs to allow vehicles to pass). The Itivia bypass road provides a bypass around Rankin Inlet from the shipping and fuel storage area in Rankin Inlet.

This report describes the geotechnical aspects of the areas inspected and presents general observations and recommendations. In addition, a description of the geophysical and permafrost conditions for the site is provided.

1.2 Scope Limitations

The scope of the inspection is limited to the observation of geotechnical aspects of each facility listed above and a review of the associated instrumentation data. The inspection did not include other assessments, such as structural, mechanical, or environmental.

2.0 INSPECTION METHODOLOGY

Each structure and its surrounding area were visually inspected for signs of settlement, seepage, cracking, distress, or permafrost degradation. Noteworthy observations were photographed and recorded. Available ground temperature cable (GTC) data, water levels, settlement monitoring data, routine monthly reports, and other relevant files and reports (listed in the reference section of this report) were reviewed. Where applicable, the inspection was performed in accordance with the principles set out in the Canadian Dam Safety Review Guidelines by the Canadian Dam Association (CDA 2013). A description of each structure follows in the subsequent sections. Drawings of the structures and photographs are in the attached appendices.

The inspection took place with no snow or ice on the lakes or land, and when surface water flows were generally low. Peak surface water flows typically occur during the freshet (May and June). During the inspection, the weather was generally sunny to cloudy. Daily temperatures ranged from 7°C to 17°C. Water levels were normal for this period of the year.

For the critical infrastructure at the Meliadine Mine (i.e., Water management infrastructure (WMI), TSF, and WRSFs), the deficiencies observed during the inspection were assessed and classified using a priority scale system developed by Agnico Eagle. A priority level was assigned to the recommendations for each critical infrastructure (see Section 17.0) for action and planning purposes.

The priority scale system provided by Agnico Eagle includes:

- **P1:**
  - A condition that compromises the safety of the structure. An action plan needs to be developed immediately and implemented as soon as possible.
- **P2:**
  - A condition that could eventually compromise the safety of the structure. An action plan needs to be developed within three months and implemented as agreed with Agnico Eagle; or
  - A serious or continued deficiency in OMS procedures, which should be resolved immediately.

- **P3:**
  - A condition that is not expected to compromise the safety of the structure, but represents an anomaly, or that is not in agreement with OMS procedures, or does not represent the best available/applicable practice. An action plan needs to be developed within six months and implemented within the period agreed with Agnico Eagle.
- **P4:**
  - A condition that is acceptable in terms of stability, serviceability, and best available/applicable practice as well as regulatory requirements, but could be modified or improved to facilitate operation, monitoring, surveillance, or aesthetics; or
  - Facilitate recommendations provided for consideration.

3.0 GENERAL SITE CONDITIONS

The Meliadine Mine is in the Kivalliq Region of Nunavut, near the northern border of the southern Arctic terrestrial eco-zone, and within the Arctic tundra climate region. It is located within the Churchill geological province, which forms part of the northern Canadian Shield.

The landscape is dominated by features characteristic of glaciated terrain and exposed bedrock. Primarily underlain by Precambrian granitic bedrock, the terrain consists of broadly rolling uplands and lowlands. The Meliadine Mine is located at an approximate elevation of 60 metres above sea level (masl) with a maximum topographic relief of 20 m. There are numerous small lakes, wetlands, and creeks, indicating poorly drained conditions. The upland areas are generally well drained. A series of low relief ridges composed of glacial deposits, oriented northwest—southeast controls the regional surface drainage pattern. Periodic ice blockages at outlets of small lakes and wetlands occur during the freshet, these can temporarily increase the downstream flood peak discharges and affect the flood characteristics. High flows are observed during the freshet, while low flows and dry stream channels are typical in late summer.

Glacial moraine deposits are predominant, ranging in thickness from veneers (less than 2 m) to blankets (2 m to 5 m) to hummocky deposits (5 m to 15 m). Glaciofluvial deposits are also present, with the most prominent being a network of sinuous eskers. Lacustrine deposits occur in association with the numerous lakes. Near the coast of Hudson Bay, finer textured marine sediments cover the ground surface.

The Meliadine Mine is in a zone of continuous permafrost and has an annual average air temperature of -9.6°C, based on climate data from Rankin Inlet for the period of record from 1994 to 2024. Within the permafrost, there are intervening taliks (areas of unfrozen ground) and thaw bulbs induced by lakes. The permafrost in the region is "cold" (i.e., has an average annual surface temperature and zero amplitude temperature of less than -4°C). The depth of the permafrost and the active layer varies with proximity to lakes, soil thickness, vegetation, climate conditions, and slope direction. Based on thermal studies and ground temperature measurements, the depth of permafrost is generally between 285 and 430 metres below ground surface (mbgs) (WSP 2024a). The depth of the active layer ranges from about 1 mbgs in areas with shallow surficial soils, up to about 3 mbgs adjacent to the lakes (Agnico Eagle 2014b). Typical permafrost ground temperatures at the depths of zero annual amplitude are in the range of-5.9°C to -7.0°C in areas away from lakes and streams, and are generally reached at a depth of 18 mbgs to 40 mbgs. The geothermal gradient ranges from 0.015°C/m to 0.02°C/m (WSP 2024a). The ground ice content in the region is expected to be between 0% and 10% (dry permafrost) based on the regional scale compilation data and the Canada Permafrost Map published by Natural Resources Canada (NRC 1993). However, areas of local higher ground ice content occur and are generally associated with low lying areas of poor drainage.

The formation of an open-talik, which penetrates the permafrost, is expected for lakes that exceed a critical depth and size. The presence and extent of each talik is influenced by the geometry (size and shape) of the lake. As the depth and size of lakes increase, the extent of the talik increases. Open taliks (defined by the 0-degree isotherm) are predicted to be present beneath portions of each of the following lakes near the proposed open pits: Lake B4, B5, B7, A6, A8, and CH6. Closed talik is interpreted below Lake D4 based on the 0-degree isotherm interpreted from the thermal model (WSP 2024b).

The salinity of groundwater also influences the temperature at which it freezes. Testing indicates that the groundwater salinity in the Mine area generally increases with depth. The mean groundwater salinity below the permafrost has been estimated at approximately 61,000 mg/L. Salinity can induce freezing point depression, creating a cryopeg in permafrost where water can remain unfrozen even when the temperature is below 0°C. The freezing point depression was calculated to be equivalent to -3.3°C (with salinity approximately 61,000 mg/L), suggesting the depth to the basal cryopeg is between 350 m and 375 mbgs in the Mine area (Golder 2012a).

The Meliadine Mine is in an area of low seismic risk and is classified as “Class C” based on ground conditions. The Peak Ground Acceleration (PGA) for a reference “Class C” site under various Annual Exceedance Probability was estimated using the 2020 National Building Code of Canada Seismic Hazard Tool. The estimated PGA is 0.0285 g for a 5% in 50-year probability of exceedance (0.001 per annum or 1 in 1,000 year return) and 0.0498 g for a 2% in 50-year probability of exceedance (0.000404 per annum or 1 in 2,475 year return) for the Meliadine Mine.

4.0 OVERALL WATER AND MINE WASTE MANAGEMENT STRATEGY

The water management objectives are to minimize potential impacts on the quantity and quality of surface water at the Meliadine Mine and surrounding waterbodies. Water management structures (culverts, sumps, pipelines, water diversion channels, and water retention dikes/berms) are utilized to contain and manage contact water from areas affected by mining activities.

Contact water originating from the Mine development areas on the surface is intercepted and diverted to various containment ponds for temporary storage. All contact water is eventually conveyed to CP1, where it is treated for total suspended solids (TSS) at the Effluent Water Treatment Plant (EWTP) and discharged through the diffuser located in Meliadine Lake.

Contact water from the Underground Mine is collected in underground sumps, transported to a clarification system, and then recirculated for use in various underground operations. Excess underground contact water is stored in temporarily inactive underground developments, and on the surface in SP1 and TIRI#2 Open Pit. Underground contact water not used for operations is treated by a reverse osmosis plant for discharge.

Waste rock and overburden are trucked to the WRSFs with distribution according to the operation schedule. Three WRSFs (i.e., WRSF1, WRSF3, and WRSF6) are constructed to accommodate the waste rock and overburden from the Mine development. Closure of the WRSFs will begin when practical as part of the progressive reclamation program. The WRSFs will not be covered and vegetated, and no additional re-grading activity will be required under the closure plan.

The tailings produced from the ore processing are filtered and mechanically placed and compacted in the TSF, and a portion of it is used underground as cemented paste backfill. The TSF consists of two cells, which are operated one after the other to facilitate progressive closure during mine operations. To manage erosion, reduce dust generation, and improve thermal performance, the side slopes of the filtered tailings are encapsulated with a 4.0 m to 4.5 m thick layer of waste rock, and the top surface of the tailings will be capped with a 0.5 m layer of overburden till overlain by a 2.5 m capping layer of waste rock under the current closure plan.

The water, waste rock, overburden, and filtered tailings were managed during the 2025 operating year (September 2024 to September 2025) in accordance with Agnico Eagle's established operating protocols, Operation, Maintenance, and Surveillance (OMS) Manuals, and management plans.

5.0 WATER COLLECTION PONDS, DIKES, AND ASSOCIATED INFRASTRUCTURE

5.1 Introduction

This section presents a summary of the water CPs and associated dikes, berms, and channels constructed prior to the 2025 annual geotechnical inspection, including:

- CP1 and its associated Dike (D-CP1) and Jetty1;
- CP2 and its associated Berm CP2, Channel9, and Channel10;
- CP3 and its associated Berm CP3, Berm2, and Channel3;
- CP4 and its associated Berm CP4 and Channel4;
- CP5 and its associated Dike D-CP5, Berm3, Jetty5, and Channel5;
- CP6 and its associated Berm CP6;
- CP9 Thermal Berm;
- Channel11;
- Berm4; and
- SP1 and SP3.

The following subsections provide a description of the structures, visual observations, a summary of geotechnical instrumentation (if any exists), followed by recommendations.

5.2 Pond CP1 and Dike D-CP1

5.2.1 Background

Dike D-CP1 was constructed across the outlets of former Lakes H6 and H17, which combine to form CP1. Construction took place between October 2016 and July 2017. The location is shown in Figure 2. Site water around the industrial facility and various collection ponds is directed to CP1. Water is retained in CP1 before TSS treatment and discharge to Meliadine Lake.

Dike D-CP1 is approximately 600 m long with a maximum height of 6.6 m (Tetra Tech 2017h). The CDA (2013) dam consequence classification for Dike D-CP1 is Significant (Tetra Tech 2016a). A downstream collection sump and two ditches were constructed approximately 5 m downstream of the D-CP1 toe to collect surface runoff and any possible dike seepage for pumping back to CP1. A thermal toe berm was constructed on the downstream side of D-CP1 in the Fall of 2021 to facilitate cooling of the dike foundation and prevent degradation of the surrounding

permafrost. In 2024, additional rockfill was placed on the east shoulder of the downstream collection ditches to reduce the potential of permafrost degradation and to provide access along the downstream collection ditches. During 2025, the access road along the downstream collection ditches was further extended to the north end of D-CP1 by placing additional rockfill cover. Additional rockfill cover was also placed at the south end of D-CP1 to facilitate the expansion of the D-CP1 pipeline crossing. Snow fences were installed across CP1 during the winter 2024/2025 to reduce snow accumulation on D-CP1. Snow removal along the downstream side of D-CP1 was also conducted regularly during the winter 2024/2025.

Selected as-built drawings, including an updated survey scan from 2025, are presented in Appendix B.

A jetty was constructed at CP1 in 2017 to pump water to the EWTP.

5.2.2 Visual Observations

The inspection of CP1, D-CP1, and associated structures was conducted on August 29, 2025, and involved walking along the crests and toes of the dike and examining the condition of the slopes for visual signs of thaw deformation, instability, cracking, and permafrost degradation. A photographic record of the inspection, with annotations added where appropriate, is included in Appendix B. The photo locations are presented in Figure 3.

During the inspection of D-CP1, the following general observations were made:

- Overall, the dike appeared stable, with no significant geotechnical concerns identified. Similar conditions have been observed since 2019.
- Erosion that primarily occurred during a high-water event between 2019 and 2020 on the upstream slope of the dike is still present, with no noticeable change since then, as shown in Photos 1 and 2 in Appendix B. The erosion has removed the finer fraction of the rockfill, leaving the larger particles. The erosion scarp is approximately 1.2 m high.
- Minor cracking and several small depression areas were observed along portions of the upstream and downstream crest (e.g., Photos 6, Appendix B). The largest cracks were up to 3 cm wide. The cracking was first observed during the 2018 geotechnical inspection and has not shown significant change since then.
- A rockfill toe berm was constructed along the downstream side of D-CP1 in the fall of 2021/2022 to facilitate cooling the dike foundation and prevent surrounding permafrost from degradation. The rockfill toe berm was placed between Stations 1+220 and 1+540 at an elevation of approximately 64.5 m. The rockfill toe berm is approximately 7 m wide. No deformation and cracking were observed on the rockfill toe berm during the 2025 annual geotechnical inspection.
- Additional rockfill was placed along the east and north perimeter of the seepage collection sump downstream of the dike in the Fall of 2021.
- The downstream toe berm appears to be performing well (Photos 7 and 8, Appendix B) with no deformation and cracking observed during the 2025 annual geotechnical inspection.
- Ponding water was observed at some depression locations along the downstream collection ditches during the 2023 annual geotechnical inspection. Regrading of the downstream collection ditches was completed in 2024 to remove localized depressions created by thaw settlement and to promote flow into D-CP1 sump (Photos 9 and 10, Appendix B). Small, localized areas of ponding water (Photo 10) were observed during the 2025 annual geotechnical inspection and are not expected to affect the dike's performance.

- The water level in the downstream collection sump was low (Photos 11 and 12, Appendix B) at the time of the 2025 annual geotechnical inspection. It is understood that water from the collection sump was pumped into CP1 during the freshet period and will be pumped out as required.
- Additional rockfill was placed along the east shoulder of the collection ditches in 2024 (Photo 13, Appendix B). The placement of additional rockfill was to prevent permafrost degradation as observed from thaw subsidence and cracking of the native ground during the 2023 annual geotechnical inspection and to provide access along the downstream collection ditches. The access road was further extended to the north end of D-CP1 in 2025 by placing additional rockfill.
- No seepage was observed along the downstream toe of Dike D-CP1.
- A white High-Density Polyethylene (HDPE) liner was installed in 2024 to cover the pipelines crossing Dike D-CP1 at Station 1+250 (Photo 14, Appendix B). The intent of the white HDPE liner is to reduce the thermal impact of the pipeline crossing on the dike performance by increasing the surface albedo and to prevent erosion of the dike material in the event of breakage of the pipelines.
- In 2025, the pipeline crossing at Dike D-CP1 was extended north from approximately Stations 1+124 to 1+134 to accommodate additional water pipelines to CP1 (Photo 15, Appendix B).
- Jetty1 was in good condition except for erosion observed on the jetty slopes. The erosion coincides with historic high-water levels in 2019/2020, as shown in Photos 16, 18, and 19, in Appendix B. The erosion is like that observed in previous years. Fines are being washed out, leaving the coarse material. The erosion is undercutting the fill up to 0.3 m in the southeast corner and may result in a slump of the surface fill in the area. The pump house is well back from the area; however, the cables in the area should be pulled back from the slope crest. Except for slope erosion, no geotechnical performance concerns were identified, and no other permafrost degradation was observed during the 2025 annual geotechnical inspection.

Agnico Eagle's engineering and environment team conduct weekly visual geotechnical inspections of the dike, pond, and ditches. Monthly inspection reports include an assessment of ground temperatures, observations of cracking and settlement, pond elevation, pumping activities, and photographs. No seepage was observed by Agnico Eagle's engineering and environmental team at Dike D-CP1 throughout the year. Agnico Eagle's staff observations were consistent with those during the 2025 annual geotechnical inspection.

5.2.3 Instrumentation and Monitoring

Horizontal and vertical GTCs were installed in D-CP1 between March and July 2017, as shown in Appendix B. Five horizontal GTCs (HGTC-1 to 5) were installed above the liner parallel to the key trench, and five vertical GTCs (VGTC-1 to 5) were installed upstream and downstream of the key trench.

The key trench temperatures are warmest in late fall (October and November) and coldest in late spring (May and June). Average key trench temperatures are summarized in Table 5-1. The mean annual air temperatures measured at Rankin Inlet (1982 to 2024) and the Meliadine Mine site from 2015 to 2025 are presented in Appendix B (Figure 4).

The following observations were made regarding the instrumentation readings for D-CP1:

- Overall, there has been a warming trend of approximately 0.3°C/year observed from 2019 to 2025, except a cooling trend (average -0.3°C) observed between 2021 and 2022. The average temperature increased by 0.51°C and 0.20°C between June 25, 2024 to June 25, 2025, and October 25, 2024 to October 25, 2025, respectively. The decrease in temperature between 2021 and 2022 could be attributed to colder-than-average air temperatures, with a below-average snowpack observed at the site during the 2021/2022 winter season. The measured air temperature at the Meliadine Mine (Appendix B, Figure 4) indicates that the mean annual air temperature has been warming up since 2018. This is likely one of the causes of the observed warming trend at Dike D-CP1 foundation. The temperatures within the key trench have remained below the thermal design target for the Dike D-CP1 foundation (i.e., -2°C) throughout the year.

- GTC data were plotted against the Thermal Performance Evaluation Model of D-CP1. The model was created in the summer of 2020 and covers the section of the dike where VGTC-03, VGTC-04, and HGTC-04 are located. The actual temperature readings from these GTCs show a slight decrease in temperature at the key trench of Dike D-CP1 between 2021 and 2023, compared to the predicted warming trend in the foundation. However, since 2024, a warming trend has been observed at the bottom of key trench by comparing to both previous years and the predicted trends. The plots illustrating actual versus modelled temperatures of D-CP1 are in Appendix B.

- Bead 11 of HGTC-1 warmed to 1.7°C in October 2020. The temperature dropped to -1.6°C in November 2020, but was still warmer than expected. It recovered to the expected temperature range in December 2020. Agnico Eagle investigated the temperature rise at the time of the occurrence. There was no ponded water near the location and no signs of infiltration. A manual reading was taken on August 30, 2024, and it was concluded that there might be an issue with the cable extension. Agnico Eagle replaced the cable extension in September 2024, and no issues have been encountered since then.

- Six settlement survey monuments were installed over the liner crest in the central area of the dike, as shown in Appendix B. Survey monitoring points M-1 to M-6 indicate a range of total vertical downward displacement between 54 mm and 104 mm since they were installed on September 19, 2017. Comparing the average displacements from 2024 to 2025, the settlement ranged from 1 mm to 10 mm along D-CP1; this settlement is minor and not expected to affect the performance of D-CP1. Most of the movement was in the first year after construction. Settlement recorded at point M-6 (Station 1+510) indicated a settlement of 49 mm between September 2021 and January 2022, with the other monitoring points showing less settlement between 9 mm to 15 mm. There were no visible signs of deformation during the inspection around point M-6. The average settlement between October 2021 and October 2022 is 14 mm. The unusual readings at M-6 were likely caused by system errors. Agnico Eagle installed a new survey control point and updated survey procedures in late 2022. The settlement data collected after 2022 still show some fluctuations but with stable trend. From the measurements taken in 2025, the fluctuations varied from 34 mm to 52 mm, these fluctuations are are most likely due to the impact of freezing and thawing of the dike's material and limitations of the survey equipment. The dike's operating water levels were based on a settlement of 120 mm; the measured settlement has been less than this to date.

Table 5-1: D-CP1 Ground Temperature Summary

Cable	Average June 5, 2019 (°C)	Average June 13, 2020 (°C)	Average June 1, 2021 (°C)	Average June 25, 2022 (°C)	Average June 25, 2023 (°C)	Average June 25, 2024 (°C)	Average June 25, 2025 (°C)	Difference June 2024 to June 2025 (C°)	Average Oct 31, 2019 (°C)	Average Oct 29, 2020 (°C)	Average Oct 27, 2021 (°C)	Average Oct 29, 2022 (°C)	Average Oct 27, 2023 (°C)	Average Oct 25, 2024 (°C)	Average Oct 25, 2025 (°C)	Difference Oct 2024 to Oct 2025 (C°)
HGTC-1	-8.4	-7.9	-8.2	-7.6	-7.4	-6.1	N/A	N/A	-4.5	-3.6	-4.2	-3.7	-3.3	-3.0	-2.9	0.1
HGTC-2	-9.2	-8.0	-7.8	-7.2	-7.1	-6.0	-5.6	0.4	-5.1	-4.8	-4.4	-4.2	-4.1	-3.6	-3.5	0.1
HGTC-3	-8.6	-7.5	-7.6	-7.5	-7.2	-6.0	-5.1	0.9	-5.6	-5.2	-5.1	-4.9	-4.9	-4.3	-4.1	0.2
HGTC-4	-8.9	-8.1	-7.9	-7.8	-7.5	-6.5	-6.1	0.4	-6.0	-5.6	-5.3	-5.0	-5.0	-4.6	-4.4	0.2
HGTC-5	-8.7	-8.2	-6.6	-7.4	-7.2	-6.0	-5.8	0.1	-3.4	-3.7	-3.9	-3.7	-3.5	-3.2	-3.2	0.0
VGTC-1	-7.2	-6.3	-5.8	-7.1	-6.9	-5.8	-5.3	0.5	-6.4	-5.4	-5.1	-5.0	-4.8	-4.3	-3.9	0.4
VGTC-2	-6.2	-5.6	-5.1	-5.5	-5.4	-4.6	-4.2	0.4	-6.1	-5.5	-4.8	-4.7	-4.7	-4.0	-3.8	0.2
VGTC-3	-7.3	-6.3	-6.2	-7.3	-7.0	-5.9	-5.4	0.5	-7.0	-6.0	-5.5	-5.5	-5.7	-4.9	-4.6	0.3
VGTC-4	-6.6	-8.1	-5.8	-6.0	-6.0	-5.3	-4.9	0.4	-6.7	-6.3	-5.4	-5.4	-5.5	-4.9	-4.7	0.2
VGTC-5	-10.3	-9.7	-9.7	-7.0	-6.9	-5.9	-5.7	0.2	-2.1	-2.1	-2.3	-2.1	-2.1	-1.6	-1.5	0.1

N/A: no data collected on June 25, 2025.

5.2.4 Water Management

CP1 receives inputs from the surrounding area and from water pumped from other areas of the site (e.g., CP3, CP4, CP5, CP6, and other sources). The design operating levels are specified in the updated OMS manual (Agnico Eagle 2024) as listed in Table 5-2.

Table 5-2: Design Water Elevations for D-CP1 Operation

Situation	Maximum Operating Water Level (m)	Requirement
End of October each year	63.7	This level is required to provide sufficient storage for: ▪ 661,500 m^3 for the runoff water from an Inflow Design Flood (IDF) event for the entire site (a total maximum catchment area of 3.675 km^2 during the design life of D-CP1); ▪ 38,800 m^3 for the treated sewage from late October to early June (8 months); and ▪ 31,000 m^3 for the treated water pumped from the SWTP to CP1 from late October to early June (8 months).
Before each spring freshet	64.1	This level is required to provide sufficient storage for: ▪ 661,500 m^3 for the runoff water from an IDF event for the entire site.
During non-IDF spring freshet or short-term after each spring freshet	66.2	This water elevation is to allow CP1 to have a storage capacity of 119,000 m^3 to store the runoff water from a 1/1,000 24-hour extreme rainfall event (77 mm precipitation) for the CP1 maximum catchment area of 1.545 km^2, without exceeding the design D-CP1 maximum water elevation of 66.6 m (under the IDF).
Short-term water elevation under the IDF	66.6	This is the design maximum water elevation for D-CP1 for a short period. The water elevation should be drawn down by pumping from CP1 to the EWTP and then discharging the treated water to Meliadine Lake.

The water level in CP1 was high over the 2019/2020 winter and was drawn down during and after the 2020 freshet. The water level has been within the normal operating range since the summer of 2020. The maximum water level was 65.1 m on June 13, 2025, and the water level at the time of the annual inspection was 63.4 m, which is below the freeze-up water level target of 63.7 m. The measured water levels in CP1 are presented in Appendix B.

5.2.5 Summary and Recommendations

CP1, Dike D-CP1, downstream seepage collection ditches and sump, and Jetty1 were generally performing well at the time of the inspection. The following recommendations are provided:

- The upstream slope of Dike D-CP1 experienced erosion in 2020 during a period of high-water levels. Surveys indicate there is 2 m of rockfill protecting the Esker Sand and Gravel in the upstream shell of the dike. The upstream slope erosion is not expected to affect the performance of the dike structure and foundation. As recommended in previous years, the performance of the upstream slope should continue to be monitored.
- It is recommended that snow management measures, including regular snow removal and the installation of snow fencing, be continued and improved to reduce snow accumulation and thereby help mitigate the observed warming trend in the dike foundation.

- It is recommended to perform a thermal performance review and calibration for D-CP1 to identify the root causes of the observed warming trend and to assess the potential consequences of the dike foundation warming up. Mitigation measures should be developed and implemented as required.
- Consideration could be given to refilling and grading the slope of Jetty1, where erosion was observed.

5.3 Pond CP2, Associated Channels, and Berms

5.3.1 Background

CP2 and its associated infrastructure (i.e., CP2 Thermal Berm, Channel9, Channel10, Channel9 Berm, and Channel10 Berm), collect and temporarily store runoff water from the WRSF3 catchment area. CP2 was created by excavating a large depression approximately 13 m deep into overburden and bedrock. CP2 Thermal Berm, located downstream of CP2, provides thermal protection to maintain the underlying permafrost. Channel9, Channel10, and their associated berms, collect and divert the runoff water from the WRSF3 catchment area. Channel9 Berm is intended to provide sufficient freeboard to Channel9 in a localized depression along the channel alignment. Channel10 Berm provides diversion of runoff into Channel10 that could otherwise potentially bypass the invert location of Channel10.

The design of CP2, Channel9, Channel10, and berms is based on the following criteria and key considerations:

- CP2 was designed to store 3/7 days of 1 in 100 wet precipitation year freshet (171 mm and assume that freshet occurs in seven days, and pumping from the facility begins three days after freshet begins).
- The maximum operating water level in CP2 under IDF is set at Elevation 52.0 m which is 2.0 m lower than the original outlet elevation of the collection pond area.
- CP2 Thermal Berm is designed to preserve permafrost in the original ground below the centre of the berms, which will minimize the potential seepage through its foundation into the downstream receiving environment (i.e., Meliadine Lake).
- The water collected in CP2 will be actively pumped into CP1 during the open water season. The intent is that CP2 will be nearly empty most of the time, except for several early days during the annual spring freshet for preparing the pump system or during an extreme rainfall event.
- Channel9 was designed to pass the design inflow during an extreme intensity flow under a 5-minute 1 in 100 return rainfall of 5 mm. Channel9 Berm is designed along Channel9 to provide sufficient freeboard and to prevent the water from overflowing the channel under the design IDF or other unexpected extreme conditions.
- Channel10 was designed to pass the design inflow during an extreme intensity flow under a 5-minute 1 in 100 return rainfall of 5 mm. Channel10 Berm positioned near the beginning of Channel10 to divert runoff from bypassing the end of Channel10 under the design IDF or other unexpected extreme conditions. The channel was constructed approximately 25 m shorter than design to prevent relocating a partially buried pipeline and electrical cable in the area. A diversion berm was constructed to ensure runoff does not flow around the end of the modified channel.

CP2 and its associated infrastructure were constructed from February 2022 to May 2022. The as-built drawings for CP2 and its associated infrastructure are included in Appendix C.

5.3.2 Visual Observations

The inspection involved walking along the crests of CP2, CP2 Thermal Berm, Channel9, Channel10, and associated berms to assess the structures for visual signs of deformation and instability, cracking, and permafrost degradation. Photos are in Appendix C. The photo locations are presented in Figure 4.

5.3.2.1 CP2 and CP2 Thermal Berm

At the time of the inspection, water in CP2 was approximately at 1/3 of its storage capacity. The slopes of the pond are a combination of overburden and bedrock. The overburden is covered with a layer of rockfill for erosion and thermal protection. The bedrock slopes are blocky with some fractured rock (Photo 1, Appendix C). No obvious signs of instability were observed in the bedrock or overburden slopes. No water flow was observed entering CP2 from the channel outlets. CP2 can be observed in Photos 2 and 3 within Appendix C.

CP2 Thermal Berm was constructed of overburden till and rockfill obtained from the excavation of CP2 and the open pit from mine operations. The till was partially frozen when it was placed in the berm. The till was covered with a layer of rockfill also obtained from the excavation. The crest of CP2 Thermal Berm appeared to be in good conditions with signs of minor settlement and cracking observed at a few locations at the time of the inspection (Photo 4, Appendix C). These minor settlements and cracking were observed during the 2023 annual geotechnical inspection and did not show deterioration over time.

The slopes of CP2 Thermal Berm were in good condition with some deformations observed at the east end of the berm's downstream side (Photos 5, Appendix C). The deformation appeared to have been caused by disturbance of the original ground during construction. The deformed area did not appear to be impacting the slope performance and overall design intent of the thermal berm.

In 2024, rockfill was placed to cover an area where surface ponding had previously been observed against the upstream toe of the CP2 Thermal Berm (Photo 6, Appendix C). During the 2025 annual geotechnical inspection, Tetra Tech did not observe signs of permafrost degradation around the CP2 Thermal Berm.

There are several areas of minor settlement at the top of the pond slopes where the rockfill cover has been placed. The settlement resulted from initial ground disturbance during the construction of CP2 and was observed during previous annual inspections. The settlement areas appear not to be impacting the slope's performance.

5.3.2.2 Channel9, Channel10, and Associated Berms

Channel9 and Associated Berm

Channel9 and its associated berm were inspected by walking along portions of their length. Overall, Channel9 and its associated berm are performing well with no noticeable geotechnical concerns identified along the channel at the time of the inspection (Photos 7 to 11, Appendix C). Several cracks and thaw subsidence areas were observed along the southern shoulder of Channel9 and the northern end of the Channel9 berm during the 2024 annual geotechnical inspection. In 2025, Agnico Eagle placed a layer of rockfill cover between Channel9 and WRSF3 to mitigate the cracking and thaw subsidence (Photos 8 and 11, Appendix C).

Channel10 and Associated Berm

Channel10 was inspected by walking along portions of its length. Overall, Channel10 and its associated berm are performing well with no noticeable geotechnical concerns identified along the channel at the time of the inspection (Photos 12 and 13, Appendix C). In 2024, several areas of thaw subsidence, depressions, and traffic-related rutting

were observed along the upstream slope of Channel 10 and on the native ground above the upstream crest. In 2025, a protective rockfill cover was placed between Channel10 and WRSF3 to prevent future thaw subsidence and address the areas of depression and rutting (Photo 14, Appendix C).

5.3.3 Instrumentation and Monitoring

Three GTCs (GTC-01, GTC-02, and GTC-03) were installed in the CP2 Thermal Berm to monitor its thermal conditions and foundation. The measured ground temperatures are shown in Appendix C. The estimated thaw depths in 2025 were 0.7 m, 1.9 m, and 1.5 m for CP2-GTC01, CP2-GTC02, and CP2-GTC03, respectively. The estimated thaw depths in 2025 are slightly shallower (0.2 m to 0.6 m) than those in 2024. The measured ground temperatures at the original ground surface (approximately 5.0 m below the berm crest) ranged from -4.3°C to -4.6°C on October 25, 2025.

Some beads stopped working at GTC-01, GTC-02, and GTC-03 during the spring and summer of 2025. The ground temperature data collected from the remaining beads is still adequate to monitor the thermal performance of the CP2 Thermal Berm.

5.3.4 Water Management

Water in CP2 was pumped out sporadically throughout the open water season through a dedicated pumping system. The water levels in CP2 between late-May 2025 and early-September 2025 varied between Elevations 46.4 m and 50.6 m, which are well below the designed maximum operating water level in CP2 under IDF (Elevation 52.0 m). The measured water levels in CP2 are presented in Appendix C.

The water level in CP2 was at approximately 47.5 m at the time of the inspection (Photos 1, 2, and 3, Appendix C). At this level, the depth of water in CP2 is approximately 3.1 m with a volume of approximately 16,150 m^3. The remaining capacity in the pond to the maximum operating level of 52.0 m is 32,010 m^3.

The inflow for the pond was based on 3/7 of the 1 in 100 wet precipitation year freshet over the catchment area of 0.43 km^2, which equates to 42,000 m^3 of water. It is understood that the water in CP2 will be drawn down to or below the freeze-up water level (45.9 m) prior to freeze up as per Agnico Eagle's water management protocols and plans.

5.3.5 Summary and Recommendations

CP2 and its associated infrastructure are performing adequately and meet the design intent based on the 2025 annual geotechnical inspection and measured data. Thaw subsidence and cracking previously observed between Channel9, Channel10, and WRSF3 were mitigated with additional rockfill placed.

5.4 Pond CP3, Associated Channels, and Berms

5.4.1 Background

CP3 and its associated infrastructure (i.e., CP3 Thermal Berm, Channel3, and Berm2), collect and temporarily store runoff water from the TSF. CP3 was created by excavating a large depression approximately 11 m deep in overburden and bedrock. The CP3 Thermal Berm provides thermal protection to maintain the underlying permafrost downstream of CP3. Channel3 collects and diverts the runoff water from the TSF catchment areas. Berm2 prevents non-contact water from flowing to the TSF catchment.

The design of CP3, Channel3, and the berms is based on the following criteria and key considerations:

- CP3 was designed to store 3/7 days of 1 in 100 wet precipitation year freshet (171 mm and assume that freshet occurs in seven days, and pumping from the facility begins three days after freshet begins).
- The maximum operating water level in CP3 under IDF is set at Elevation 63.0 m, which is 2.0 m lower than the original outlet elevation of the collection pond area.
- CP3 Thermal Berm is designed to preserve permafrost in the original ground below the centre of the berms, minimizing the potential for seepage through its foundation into the downstream receiving environment (i.e., Lake B7).
- The water collected in CP3 will be actively pumped to former Lake H13, which flows into CP1 during the open water season via various culverts and Channel1. The intent is that CP3 will be nearly empty most of the time, except for several early days during the annual spring freshet for preparing the pump system or during an extreme rainfall event.
- Channel3 was designed to pass the design inflow during an extreme intensity flow under a 5-minute 1 in 100 return rainfall of 5 mm. A berm incorporated into the CP3 access road was designed along Channel3 to provide sufficient freeboard and to prevent the water from overflowing the channels under the design IDF or other unexpected extreme conditions.
- Channel3 is located along the southwestern boundary of the TSF and directs seepage and runoff water from the TSF into CP3. Channel3 is approximately 620 m long with a designed base width of 1 m to 2 m.

CP3 and its associated infrastructure were constructed from August 2018 to January 2019. The as-built drawings for CP3 and its associated infrastructure are included in Appendix D.

5.4.2 Visual Observations

The inspection involved walking along the crests of CP3, Berm2, Channel3, and CP3 Thermal Berm to assess the structures for visual signs of deformation and instability, cracking, and permafrost degradation. Photos are in Appendix D. The photo locations are presented in Figure 5.

5.4.2.1 CP3 and CP3 Thermal Berm

Some water was observed in CP3 during the inspection. The slopes of the pond are a combination of overburden and bedrock. The bedrock slopes are blocky with some fractured rock. The overburden slopes are covered with a layer of rockfill placed. No obvious signs of instability were observed in the bedrock or overburden slopes. As observed in 2023 and 2024, portions of the slope were covered with sediment eroded from an area of disturbed ground east of CP3 (Photo 1, Appendix D). A layer of rockfill was placed between the toe of the TSF and CP3 in the summer of 2023 to serve as thermal protection (Photo 2, Appendix D). A snow stockpile was observed between the toe of the TSF and CP3 Thermal Berm (Photo 3, Appendix D).

CP3 Thermal Berm was constructed from overburden till and rockfill obtained from the excavation of CP3 in 2019. The till was partially frozen when it was placed in the berm. The till was covered with a layer of rockfill also obtained from the excavation. The slopes of the thermal berm were in good condition at the time of the inspection. The crest of CP3 Thermal Berm is undulating due to settlement that occurred as shown in Photo 4, Appendix D. The settlement appears not to impact the berm's function which is to preserve the permafrost below the original ground. Signs of permafrost degradation were not observed around CP3 Thermal Berm at the time of the inspection.

5.4.2.2 Channel3 and Berm2

Channel3 was initially constructed in 2019 to divert runoff from the catchment area of the TSF towards CP3. Agnico Eagle reconstructed Channel3 to its design grade in the winter of 2022/2023, following performance concerns raised and recommendations provided in previous annual geotechnical inspections. The channel was inspected by walking along its entire length. Overall, the reconstructed Channel3 is performing well; no water flow was observed, and no geotechnical concerns were identified at the time of the inspection. It was noticed that the original ground between the TSF and Channel3 was covered by rockfill material during the reconstruction of Channel3. This additional rockfill will provide additional thermal protection and reduce the potential for permafrost degradation. Channel3 is shown in Photos 5 to 8, in Appendix D.

Berm2 was constructed to reduce the amount of non-contact water entering the TSF and the CP3 catchment areas, as shown in Photos 9 to 11, in Appendix D. Berm2 was predominantly constructed of 50 mm minus screened esker material with a till zone approximately 2 m wide. At the time of the inspection, Berm2 was retaining water in a low spot along its length. There was minimal water on the downslope side of the berm, indicating that the berm is functioning as intended. Cracking along the crest and slope of the berm was repaired by re-grading and compaction using an excavator in 2024.

Cracking and minor erosion were observed along the crest and slope of the berm, similar to conditions observed before the berm was repaired in 2024. The overall performance of Berm2 is adequate as designed.

5.4.3 Instrumentation and Monitoring

Three GTCs (GTC-01, GTC-02, and GTC-03) were installed in the CP3 Thermal Berm to measure the active layer depth and subgrade ground temperatures. The measured ground temperatures are shown in Appendix D. Based on the ground temperatures collected in 2025, the estimated active layer thickness varied from 1.9 m to 3.3 m on October 25, 2025. The measured ground temperatures at the original ground surface (approximately 5 m to 6 m below the berm crest) ranged from -2.4°C to -2.9°C on October 25, 2025.

5.4.4 Water Management

Water was pumped out sporadically throughout the open water season through a dedicated pumping system. Water levels in CP3 between mid-May 2025 and early-September 2025 ranged from 55.5 m to 62.73 m. The measured water levels in CP3 are presented in Appendix D.

The water level at the time of the inspection was 56.1 m. At this level, the depth of water in CP3 is approximately 2.1 m with a volume of approximately 4,320 m^3. The remaining capacity in the pond to the maximum operating level of 63.0 m is 40,530 m^3 based on the as-built CP3 geometry.

The inflow for the pond was based on 3/7 of the 1 in 100 wet year precipitation freshet (171 mm) over the catchment area of 0.383 km^2 which equates to 28,000 m^3 of water. It is understood that the pond will be pumped to near empty prior to freeze up as per Agnico Eagle's water management protocols and plans.

5.4.5 Summary and Recommendations

CP3 and its associated infrastructure are performing adequately.

As per design, CP3 needs to be drained below the freeze-up target water level at the end of each fall. The pond base is irregular, making complete drainage difficult. The pond's minimum elevation is 54.0 m. Agnico Eagle specified a minimum drawdown level of 57.47 m before freeze-up. At this elevation, approximately 10,300 m^3 would

remain in the pond. The as-built volume of CP3 provides 14,675 m^3 of contingency storage at the maximum operating level of 63.0 m, so the drawdown target is not expected to affect the pond's design intent.

CP3, CP3 Thermal Berm, Berm2, and the reconstructed Channel3 are functioning as designed.

5.5 Collection Pond CP4, Associated Channels, and Berms

5.5.1 Background

CP4, and its associated infrastructure, CP4 Thermal Berm, and Channel4, collect and temporarily store runoff water from the catchment of WRSF1. CP4 was created by excavating a large depression approximately 15 m deep in overburden and bedrock. The CP4 Thermal Berm, downstream of CP4, provides thermal protection to maintain the underlying permafrost. Channel4 collects and diverts the runoff water from the WRSF1 catchment area.

The design of CP4, CP4 Thermal Berm, and Channel4 is based on the following criteria and key considerations:

- CP4 was designed to store 3/7 days of 1 in 100 wet precipitation year freshet (171 mm and assumes that freshet occurs in seven days and pumping from the pond occurs after day three). The excess freshet water will be pumped out to partially drained Lake H13 during the freshet period, from which it will flow to CP1 through culverts and Channel1.
- The maximum operating water level in CP4 under IDF is set at Elevation 63.0 m, which is 2.0 m lower than the original outlet elevation of the collection pond area.
- CP4 Thermal Berm, is designed to preserve permafrost in the original ground below the centre of the berm, thereby minimizing potential seepage through its foundation into the downstream receiving environment (i.e., Lake B7).
- The water collected in CP4 will be actively pumped to former Lake H13, which flows into CP1 during the open water season. The intent is that CP4 will be nearly empty most of the time, except for several early days during the annual spring freshet, when the pump system will be prepared, or during an extreme rainfall event.

CP4 and its associated infrastructure were constructed from October 2018 to May 2019. A rockfill berm was constructed along the downstream shoulder of Channel4 (south side) in the summer of 2024 to provide traffic access and improve the flow capacity at the end of the channel. The as-built drawings for CP4 are included in Appendix E.

5.5.2 Visual Observations

The inspection involved walking along the crests of CP4, Channel4, the Channel4 downstream access rockfill berm, and the CP4 Thermal Berm to assess the structures for visual signs of deformation and instability, cracking, and uneven surfaces. Photographs of CP4 and the associated infrastructure are in Appendix E. The photo locations are presented in Figure 6.

5.5.2.1 CP4

At the time of inspection, CP4 was filled with water to approximately Elevation 54.9 m. The pond slopes are a combination of overburden and bedrock. The overburden is covered with a layer of rockfill obtained from the pond excavation. During the 2021 inspection, thaw settlement up to 0.75 m deep was observed in the native ground above the overburden slope protection rockfill along the west and south sides of CP4. The native ground above the

overburden slope had been covered with a protective layer of rockfill along the west and south sides of CP4 prior to the 2022 inspection to prevent additional thaw settlement. Additional rockfill was placed along the east and north sides of CP4, and in the area between CP4 and WRSF1, prior to the 2023 inspection, to serve as additional thermal protection. No obvious signs of instability were observed in the bedrock or overburden slopes (Photos 1 to 3, Appendix E).

5.5.2.2 CP4 Thermal Berm

CP4 Thermal Berm was constructed of overburden till obtained from the excavation of CP4. The till was a combination of frozen and unfrozen material when it was placed in the berm. The till was covered with a layer of rockfill also obtained from the excavation. The crest of the berm was regraded in 2024, to remediate settlement observed in previous years, Photos 4 and 5, Appendix E. The crest and slopes of the berm were in good condition (Photos 6 to 8, Appendix E) at the time of the inspection. No noticeable cracking or signs of instability were observed during the inspection, and permafrost degradation was not observed.

5.5.2.3 Channel4

Channel4 was constructed to divert runoff from the WRSF1 catchment area into CP4. The as-built side slopes range from 3.5H:1.0V to1.8H:1.0V, and the channel base varies from 0.8 m to 3.3 m wide. Channel4 is shown in Photos 8 to 13, Appendix E. No water was flowing in the channel at the time of the inspection; however, there were localized areas of shallow ponded water due to an uneven channel base. It appears there has been some thaw subsidence in the base of the channel. The thaw subsidence areas observed in 2022, where the channel ties into the native subgrade east of the channel, were covered with a protective layer of rockfill in 2024 to reduce further thaw subsidence and erosion between the channel and WRSF1. After placement of the rockfill cover, no further thaw subsidence or erosion has been observed.

A recommendation was made during the 2024 annual geotechnical inspection to place rockfill cover material between Channel4 and WRSF1 as thermal and erosion protection cover. During the 2025 annual geotechnical inspection, Tetra Tech observed that a layer of rockfill material was actively being placed between Channel4 and WRSF1 (Photo 9, Appendix E).

Surface erosion at the downstream end of Channel4 Berm near Station 0+620 was observed during the 2023 annual geotechnical inspection. In the summer of 2024, a rockfill and till berm was constructed on the downstream shoulder of Channel4 to remediate erosion and improve channel capacity (Photos 11 and 12, Appendix E), no further surface erosion was observed at this location of the channel during the 2025 annual geotechnical inspection. Overall, Channel4 is performing adequately and in accordance with its design intent.

5.5.3 Instrumentation and Monitoring

Two GTCs (GTC-01 and GTC-02) were installed in the CP4 Thermal Berm to measure the active layer depth and subgrade ground temperatures in the berm. The ground temperature profiles from these GTCs are shown in Appendix E. The estimated active layer thickness varied from 2.5 m to 3.0 m based on the measured temperature on September 25, 2025. The ground temperature at the original ground surface (approximately 5 m below the berm crest) ranged from -3.5°C to -3.7°C on October 25, 2025.

5.5.4 Water Management

Water levels in CP4 from mid-May 2025 to early-September 2025 ranged from 53.3 m to 59.7 m. At the time of inspection, the water level in CP4 was 54.9 m, resulting in approximately 2.3 m of water in the pond. The measured water levels in CP4 are presented in Appendix E.

As of August 29, 2025, the remaining capacity (to the maximum operating level of 63.0 m) was 41,830 m^3. The inflow for the pond was based on 3/7 of the 1 in 100 year freshet (171 mm) over the catchment area of 0.441 km^2 which equates to 32,300 m^3 of water. It is understood that the pond will be pumped to near “dry” condition prior to freeze up as per Agnico Eagle’s water management protocols and plans.

5.5.5 Summary and Recommendations

CP4 and its associated infrastructure are performing adequately. The thaw settlement of the native ground above the rockfill protected overburden slope of CP4 observed during the 2021 annual geotechnical inspection has been covered with a protective layer of rockfill along the west and south sides of CP4 to reduce future thaw subsidence in the area. The till berm between CP4 and the upstream slope of CP4 Berm has also been covered with a minimum of 1.5 m rockfill to reduce future settlement and ponding on the surface of the till berm. Additional rockfill was placed along the east and north sides of CP4 and the area between CP4 and WRSF1 prior to the 2023 annual geotechnical inspection to provide additional thermal protection

The operating procedure for the pond specifies that CP4 be completely drained prior to freeze-up. However, the base of CP4 is irregular, making complete drainage difficult. The minimum elevation of CP4 is 52 m. Agnico Eagle specified that operations target a minimum drawdown level of 55.28 m prior to freeze-up. At this elevation, approximately 8,300 m^3 of water would remain in CP4, which is not expected to compromise the storage capacity during the IDF event. The as-built volume of CP4 provides 15,375 m^3 of contingency storage at the maximum operating level of 63.0 m, therefore the drawdown target is not expected to adversely affect the design intent of the pond.

CP4 Thermal Berm is performing adequately and as designed. The measured ground temperature data indicates that the original ground below CP4 Thermal Berm is in a frozen condition. No cracking or signs of instability were observed during the inspection. No signs of significant permafrost degradation were observed around the CP4 area at the time of the inspection.

Channel4 is also performing adequately as designed. Surface erosion observed previously was remediated in 2024 by placing rockfill cover on the downstream shoulder of Channel4. Localized thaw subsidence, cracking, and settlement of overburden material were observed at several locations along the upstream shoulder of Channel4 during the previous annual geotechnical inspections. These features observed indicate localized thermal degradation and are likely related to surface runoff flow over the area between WRSF1 and Channel4. During the 2025 annual geotechnical inspection protective rockfill placement was observed between Channel4 and WRSF1 to mitigate thaw subsidence and cracking.

Localized ponding of water was also observed at the bottom of Channel4. Based on the size of the ponded water area, the overall performance and functionality of Channel4 are not expected to be affected.

5.6 Pond CP5 and Dike D-CP5

5.6.1 Background

Dike D-CP5 was constructed between October 2016 and July 2017 across the south portion of former Lake A54. The intent of D-CP5 is to form CP5 in the north portion of former Lake A54.

D-CP5 is approximately 300 m long with a maximum height of 3.3 m (Tetra Tech 2017g) and is located north of the TIRI#2 Open Pit as shown in Figure 2. Dike D-CP5 is classified as Significant based on the dam consequence classification recommended in the CDA (2013) (Tetra Tech 2016b). CP5 is used seasonally for temporary water storage, with active pumping to CP1 to transfer the water out of CP5.

The access road to the TIRI#2 Open Pit has been constructed downstream of the dike. The area between the dike and the road has been graded with crushed rock, covering the seepage collection pond located downstream of the dike. The road constructed downstream of the dike could provide some benefits to help maintain the frozen condition of the foundation below the dike key trench.

5.6.2 Visual Observations

The inspection involved walking along the crests and toes of the dike and examining its slopes for visual signs of deformation and instability, cracking, and uneven surfaces on August 29, 2025. A photographic record of the inspection is included in Appendix F. The photo locations are presented in Figure 7.

At the time of the inspection of D-CP5, the following general observations were made:

- Overall, the dike appeared stable, with no geotechnical concerns identified.
- Minor cracking was observed at a few locations on the upstream and downstream sides of the dike crest. The cracking appeared consistent with that observed in 2021 and did not appear to be progressing. The dike crest is shown in Photos 1 to 4, Appendix F.
- There were no signs of seepage from the downstream toe.

Jetty5 is the causeway for the pump back station for CP5. A water diversion ditch was excavated in 2024 around the jetty to divert water from localized areas to the jetty. Minor slope erosion was observed at several locations along the ditch due to the excavation. Given the distance between the ditch and jetty, it is not expected that the performance of the jetty will be impacted by the ditch.

Agnico Eagle's environment team conducts weekly visual geotechnical inspections of the dike. Monthly inspection reports include assessment of ground temperatures, observations of cracking and settlement, pond elevation, pumping activities, and photographs. The observations made by Agnico Eagle staff were consistent with the observations during the 2025 annual geotechnical inspection. Cracks and settlement locations were marked with spray paint in the field to monitor changes (Photo 4, Appendix F). No permafrost degradation was observed at the D-CP5 and CP5 areas.

5.6.3 Instrumentation and Monitoring

Horizontal and vertical GTCs were installed in D-CP5 between March and July 2017. Plots of the ground temperature data are provided in Appendix F. Two horizontal GTCs (HGTC-1 and HGTC-2) were installed in

D-CP5 above the liner parallel to the key trench, and three vertical GTCs (VGTC-1 to 3) were installed upstream and downstream of the key trench.

Key trench temperatures are warmest in late fall (October and November) and coldest in late spring (May and June). The average temperatures along the portion of the cable in the key trench parallel to the dike axis are summarized in Table 5-3 at specific dates.

The horizontal GTCs indicate a slight cooling trend with an average change of -0.19°C and -0.01°C in the base of the key trench from May 27, 2024 to May 25, 2025, and from September 25, 2024 to September 25, 2025, respectively. The vertical GTCs indicate a slight warming trend average change of 0.48°C and 0.26°C in the foundation of the dike from May 27, 2024 to May 25, 2025, and from September 25, 2024 to September 25, 2025, respectively. The temperatures within the key trench have remained below the thermal design target for the Dike D-CP5 foundation (i.e., -1.5°C) throughout the year.

Three settlement survey monuments were installed over the liner crest in the dike. D-CP5 survey monitoring points indicate a settlement between 6 mm and 47 mm since installation. Comparing the average displacements from 2024 to 2025, the settlement was 1 mm at each survey monument along the D-CP5, this settlement is minor and not expected to affect the performance of D-CP5. Agnico Eagle installed new survey control points and updated their survey procedure in late 2022. There is “noise” in the readings, but improvements have been made to stabilize the settlement readings. The settlement data show some fluctuations but with a stable trend. From the measurements taken in 2025, the fluctuations varied from 13 mm to 24 mm, but on a stable trend. These fluctuations are mostly due to the impact of freezing and thawing of the dike’s material and limitations of the survey equipment and it is not a indicator of significant settlement. The dike operating water levels were based on a settlement of 100 mm; the measured settlement has been less than this to date. The settlement data is provided in Appendix F.

Table 5-3: D-CP5 Ground Temperature Summary

Cable	Average June 4, 2019 (°C)	Average May 31, 2020 (°C)	Average May 31, 2021 (°C)	Average May 25, 2022 (°C)	Average May 25, 2023 (°C)	Average May 27, 2024 (°C)	Average May 25, 2025 (°C)	Difference May 2024 to May 2025 (C°)	Average Oct 31, 2019 (°C)	Average Oct 29, 2020 (°C)	Average Oct 27, 2021 (°C)	Average Sept 29, 2022 (°C)	Average Sept 25, 2023 (°C)	Average Sept 25, 2024 (°C)	Average Sept 25, 2025 (°C)	Difference Sept 2024 to Sept 2025 (C°)
HGTC-1	-7.8	-7.7	-7.0	-8.2	-8.1	-6.6	-7.0	0.4	-2.2	-2.3	-2.3	-2.8	-2.8	-2.5	-2.5	0.0
HGTC-2	-8.0	-8.0	-7.3	-8.4	-8.2	-6.9	-7.0	0.1	-2.9	-2.8	-2.9	-3.3	-3.2	-3.0	-3.0	0.0
VGTC-01	-4.3	-4.7	-4.6	-5.8	-6.0	-5.1	-4.2	0.9	-3.6	-3.8	-3.3	-4.0	-4.1	-3.6	-3.3	0.3
VGTC-02	-4.6	-5.2	-5.0	-5.6	-6.8	-5.5	-5.0	0.5	-3.8	-3.9	-3.5	-4.0	-4.2	-3.7	-3.3	0.4
VGTC-03	-3.3	-3.5	-3.3	-6.0	-5.9	-5.3	-5.3	0.0	-3.3	-3.6	-3.4	-4.1	-4.8	-3.9	-3.9	0.0

5.6.4 Water Management

CP5 collects runoff from the surrounding catchment area. Water in CP5 is actively pumped to CP1 throughout the open water season. The design operating levels are specified in the updated OMS manual (Agnico Eagle 2024) as listed in Table 5-4.

Table 5-4: Design Water Elevations for D-CP5 Operation

Situation	Maximum Operating Level (m)	Requirement
Before and after each spring freshet	65.5	This water elevation was determined to allow CP5 to have a sufficient storage capacity to store the estimated maximum volume of 49,500 m^3 of the runoff water from an IDF event for a total maximum CP5 catchment area of 0.643 km^2 during the design life of D-CP5, which includes the catchment areas of the P1/P2/P3 and Portal No. 1 areas.
During mean spring freshet (assumed to store 3 of 7 days of spring freshet)	66.03	This water elevation was determined to store 3/7 of the runoff water from a mean spring freshet for the total maximum CP5 catchment area of 0.643 km^2.
Under the IDF	66.32	This is the design maximum water elevation for D-CP5 for a short period. The water elevation should be drawn down to 64.8 m by pumping water to CP1 after each spring freshet or rainfall event; and This water elevation is also constrained by the risk of flooding Portal No. 1, the nearby ventilation shaft, and the saline water storage pond.

The water level in CP5 varied from 65.0 m to 65.8 m from mid-May 2025 to early-September 2025 which is within the operating levels of the pond. On August 29, 2025 the water level was at Elevation 65.0 m which is below the target water elevation prior to freeze up. The measured water levels in CP5 are presented in Appendix F.

5.6.5 Summary and Recommendations

Dike D-CP5 and the associated infrastructure are in good condition. No concern have been identified at this stage.

5.7 Collection Pond CP6 and Associated Berm

5.7.1 Background

CP6 collects and temporarily stores runoff water from the waste rock storage area (WRSF3). CP6 was created by excavating a large depression approximately 7 m to 11 m deep in overburden and bedrock. CP6 Thermal Berm, downstream of CP6, provides thermal protection to maintain the underlying permafrost within the footprint of the CP6 Thermal Berm.

The design of CP6 and CP6 Thermal Berm is based on the following criteria and key considerations:

- CP6 was designed to store 3/7 days of 1 in 100 wet precipitation year freshet (171 mm and assumes that freshet occurs in seven days and pumping from the pond occurs after day three). The excess freshet water will be pumped to CP1.

- The maximum operating water level in CP6 under IDF is set at Elevation 60.0 m, which is 2.0 m lower than the original outlet elevation of the collection pond area.
- CP6 Thermal Berm is designed to preserve permafrost in the original ground below the centre of the berm, which will minimize the potential seepage through its foundation into the downstream receiving environment.
- The water collected in CP6 will be actively pumped to CP1. The intent is that CP6 will be nearly empty most of the time, except for several early days during the annual spring freshet for preparing the pump system or during an extreme rainfall event.

CP6 and CP6 Thermal Berm were constructed from March 2020 to April 2020. The as-built drawings for CP6 are included in Appendix G.

5.7.2 Visual Observations

The inspection involved walking along the perimeter of CP6, and the crest and slopes of CP6 Thermal Berm to examine the structures for visual signs of deformation and instability, cracking, permafrost degradation, and uneven surfaces. Photographs of CP6 and CP6 Thermal Berm are in Appendix G. The locations of the photos are presented in Figure 8. Observations are summarized below:

- At the time of inspection, the water level in CP6 was far below the top of the bedrock. The slopes of the pond are a combination of overburden and bedrock. The overburden is covered with a layer of rockfill obtained from the pond excavation. No obvious signs of instability were observed in the bedrock or overburden slopes (Photos 1 to 4, Appendix G).
- In 2023, the access ramp into CP6 was extended to the base of the pond to provide operations with safe access for dewatering, the ramp is still functioning as intended with no geotechnical concerns (Photo 2, Appendix G).
- In 2024, the area between the CP6 access ramp and CP6 Thermal Berm was filled with rockfill to eliminate ponding water, no water or erosion was observed at this area during the 2025 annual geotechnical inspection. In 2025, rockfill was placed between CP6, WRSF3, and Channel9 to mitigate thaw settlement, minor cracking, and subsurface erosion previously observed along the east side of the CP6 perimeter (Photos 5 to 7, Appendix G). Rockfill was also placed over an area of depression observed in previous years between CP6 (south perimeter) and WRSF3.
- The CP6 Thermal Berm was constructed of overburden till obtained from the excavation of CP6. The till was a combination of frozen and unfrozen material when it was placed in the berm. The till was covered with a layer of rockfill also obtained from the excavation. The slopes of the berm were in good condition (Photos 8 and 9, Appendix G). Minor cracks and localized settlement areas were observed on the crest of the berm (Photo 8, Appendix G). The cracks and settlement were initially observed in the following summer after the construction of the berm and do not appear to be impacting the berm's function which is to preserve the permafrost below the original ground surface.

5.7.3 Instrumentation and Monitoring

Three GTCs were installed in the CP6 Thermal Berm to measure the active layer depth in the berm and subgrade ground temperatures. GTC-02 stopped reading after the last measurement was taken on May 25, 2022. The remaining two GTCs are adequate to monitor the thermal performance of the berm. The GTCs are shown in Appendix G. Based on the ground temperature data collected on October 25, 2025, the estimated thawed depth was approximately 1.9 m to 2.7 m. The ground temperature at the original ground surface (approximately 5 m to 6 m below the berm crest) ranged from -2.9°C to -4.2°C on November 25, 2025.

5.7.4 Water Management

Water levels in CP6 from late-May 2025 to early-September 2025 ranged from 52.9 m to 57.4 m. The water level was at approximately 53.1 m during the inspection, resulting in approximately 1.6 m depth of water in the pond. This equates to approximately 3,400 m^3 of water within CP6 based on the storage curve. Water was pumped out sporadically throughout the open water season. The measured water levels in CP6 are presented in Appendix G.

As of August 29, 2025, the remaining capacity (to the maximum operating level of 60.0 m) was 42,480 m^3. The inflow for the pond was based on 3/7 of the 1 in 100 freshet (171 mm) over the catchment area of 0.448 km^2, which equates to 32,696 m^3 of water.

5.7.5 Summary and Recommendations

Generally, CP6 and CP6 Thermal Berm are performing well. Additional rockfill was placed in 2025 to mitigate a small amount of thaw settlement, minor cracking, and subsurface erosion at the east side of CP6. This rockfill cover is performing well, and no additional settlement, cracking, or erosion in the area was observed during the 2025 annual geotechnical inspection.

5.8 CP9 and Associated Water Management Infrastructure at Pump

5.8.1 Background

The water management infrastructure (WMI) at pump includes CP9, Channel11, Berm4, and CP9 Thermal Berm. The construction of Channel11, Berm4, and CP9 Thermal Berm took placed February 2025 to May 2025. CP9 will be formed by Pump01 open pit following the completion of mining and the placement of thermal cover at Pump01. At the time of the 2025 annual geotechnical inspection, the mining at the Pump01 open pit was ongoing, and the mining completion date at the Pump01 open pit is unknown.

The purpose of Channel11 is to collect and divert the runoff water from the WRSF6 catchment area to CP9. CP9 will be used to collect and store runoff water from WRSF6 and adjacent mining areas and serve as one of the temporary water storage areas at Pump during operation. The CP9 Thermal Berm is to preserve the permafrost foundation within the CP9 Thermal Berm footprint and to prevent seepage from Lake B4 to Pump01 open pit. Berm4 is to divert runoff water from WRSF6 to Channel11, and to prevent contact water from flowing into the outside receiving environment (e.g., Lakes B45 and B59).

The design of CP9, Channel11, Berm4, and CP9 Thermal Berm is based on the following criteria and key considerations:

- Contact water under IDF (i.e., full equivalent unit runoff during a 1 in 100 wet year spring freshet) will be stored in CP9 under the designed maximum operating water level without pumping out during the spring freshet. The maximum operating water level under IDF is set to be minimum 2.0 m lower than the outlet of the pond (the lowest ground elevation (i.e., 59 m) along the perimeter of Pump01 open pit).
- A water pumping system will be installed in CP9 to manage the water. The pumping rate was designed to pump out the entire runoff volume generated under an IDF event and total rainfall under a mean precipitation condition over 81 days (from mid-June to mid-September) with 10 days of contingency. Under an extreme wet year scenarios (i.e., IDF plus total annual rainfall under a 1 in 100 wet year precipitation), excess runoff water can be managed by adaptive water management measures including water transfer to other storage areas, extending the pumping period, and/or installation of temporary pumping systems as needed.

- During normal operations, the water level in CP9 will be maintained below the top elevation of bedrock. However, it is permissible to allow the water level to be maintained in the overburden zone for temporary storage, if there are resource limitations or other operational constrains during the freshet period.
- Before freeze-up, water will be pumped out, and only minimal water will be left in CP9 to provide sufficient storage for the runoff generated in the following year's freshet. The freeze-up target water level is determined by the difference in volume between the maximum operating water level and the volume to be stored during the IDF event, plus 50% contingency (for snow accumulation and other unforeseen events).
- Channel11 was designed to pass the design inflow during an extreme intensity flow under a 5-minute 1 in 100 return rainfall of 5 mm with a freeboard of 0.2 m.
- CP9 Thermal Berm, was designed to to preserve the permafrost foundation within the CP9 Thermal Berm footprint and to prevent seepage from Lake B4 to Pump01 open pit.
- The design of Berm4 includes a low permeability overburden till core for seepage control, and rockfill capping for the erosion protection.

The as-built drawings for the CP9 WMI are included in Appendix H.

5.8.2 Visual Observations

The inspection involved walking along the crests of CP9 Thermal Berm, Channel11, and Berm4 to examine the structures for visual signs of deformation and instability, cracking, and uneven surfaces. At the time of the inspection, mining at Pump01 open pit and the placement of thermal cover over the overburden slope were still ongoing. Photographs for the CP9 WMI are in Appendix H. The photo locations are presented in Figure 9.

5.8.2.1 CP9 Thermal Berm

CP9 Thermal Berm was constructed of overburden till obtained from the stripping of Pump01 open pit. The till consisted of frozen material when it was placed in the berm and covered with a layer of rockfill also obtained from the open pit excavation. The slopes of the berm were in good condition (Photos 1, 2, and 5, in Appendix H) at the time of the inspection. The crest of the berm was performing well; minor cracking was observed at both the upstream and downstream crests at one location along the berm (Photos 2 and 4, Appendix H). No other cracks or signs of instability, and permafrost degradation were observed during the inspection.

Ponding water was observed, on both downstream and upstream sides of the berm during the inspection (Photos 5 and 6, in Appendix H). The ponding water located on the downstream side was in a small, localized area and appeared to be from a depression created during construction, as grubbed soil was observed around the pond. The ponding water on the upstream side of the berm is located at the north end and appears to be from a depression created by the natural topography and rockfill apron between CP9 and CP9 Thermal Berm. It is recommended that the ponding water on the upstream side be monitored regularly to determine whether mitigation measures (e.g. backfilling with rockfill or actively pumping to CP9) are required. No seepage was observed at the thermal berm during the 2025 annual geotechnical inspection.

5.8.2.2 Channel11

Channel11 was constructed out of layers of clean rockfill, non-woven geotextile, and riprap. The as-built side slopes range from 3.0H:1.0V to 2.3H:1.0V, with the base of the channel varying from 1.6 m to 2.0 m wide. The channel was inspected by walking along its entire length. The slopes of the channel were in good condition, as shown in Photos 7 to 10, in Appendix H. Overall, Channel11 is performing well; no water flow was observed, and no

geotechnical concerns were identified along the channel at the time of the inspection. Some minor cracking was observed, and settlement was starting to occur between WRSF6 and Channel11 (Photo 11, Appendix H). The cracking observed is likely from the settlement of a thin layer of granular material left between the channel and WRSF6 after the construction of Channel11. This cracking and settlement are not expected to impact the performance of Channel11; therefore, no mitigation is required at this stage.

5.8.2.3 Berm4

Berm4 was constructed of overburden till and clean rockfill from the excavation of CP9. A till core, which was approximately 3.0 m wide, was placed at the crest and then subsequently covered with a layer of rockfill material which is shown in Photos 12 to 14, in Appendix H. At the time of the inspection, ponded water was observed at the southeast end of Berm4, between the berm and WRSF6, and a depression was observed in the upstream rockfill slope of Berm4 adjacent to the ponding water (Photo 14, Appendix H). The ponding water is likely due to the depression caused during the construction and localized high topography that prevents the water from flowing to Channel 11. No water was observed on the downstream side of the berm indicating that the berm is functioning as intended.

Overall Berm4 is performing well. It is recommended that the areas of ponding water between Berm4 and WRSF6 are monitored to determine whether backfilling is required.

5.8.3 Instrumentation and Monitoring

Three GTCs (GTC25-01, GTC25-02, and GTC25-03) were installed in the CP9 Thermal Berm to measure the active layer depth in the berm and subgrade ground temperatures. The GTCs are shown in Appendix H. The estimated thaw depth on October 25, 2025 is approximately 1.5 m. The ground temperature at the original ground surface (approximately 5.5 m below the berm crest) ranged from -3.8°C to -5.2°C on October 25, 2025. Based on ground temperature data and visual observations, the CP9 Thermal Berm is performing well, and the original ground beneath the berm remains frozen.

5.8.4 Summary and Recommendations

CP9 Thermal Berm, Channel11, and Berm4 are performing adequately.

Localized cracking was observed at one location along the crest of the CP9 Thermal Berm, and ponding water was observed along the downstream and upstream toes of the berm. The cracking and the downstream ponding water should be monitored and are not expected to affect the performance of the berm. The areas of ponding water against the upstream portion of CP9 Thermal Berm and between WRSF6 and Berm4 should be monitored to determine whether backfilling or pumping is required.

6.0 SALINE PONDS

6.1 Saline Pond1

SP1, which is located north of CP5, was constructed during the third quarter of 2016 to manage underground saline water.

SP1 was constructed by excavating permafrost overburden and bedrock. A small berm approximately 1 m to 2 m high was constructed around the excavation, using a till core and rockfill cover, to promote the development of foundation permafrost in the original ground below the berm and to prevent surface water from the surrounding area from draining into the pond. The pond is designed to maintain the maximum pond elevation under the IDF (1-in-100-year wet precipitation event) below the original ground and the maximum operating water level in CP5 to minimize the potential for seepage from the saline pond.

The inspection involved walking along the crest of the saline pond's perimeter berm to examine the berm for visual signs of deformation and instability, cracking, uneven surfaces, permafrost degradation, and seepage. A selection of photographs from the inspection is included in Appendix I. The photo locations are presented in Figure 10.

At the time of the inspection of SP1, the following general observations were made:

- Overall, the conditions of SP1 and the perimeter berm appeared stable, as observed during the 2024 annual geotechnical inspection.
- There was no seepage observed on the wall of SP1.
- Water in SP1 at the time of the inspection was well below the top of the bedrock excavation (Photo 1, Appendix I).
- The thermal berm appeared to be in good condition with minimal cracking (Photos 2 to 5, Appendix I).
- The southwest corner of the pond had significant cracks up to 100 mm wide at the crest (Photos 4 and 5, Appendix I). The slopes below the cracking may be deformed. The cracks could be due to thaw subsidence or movement of the overburden slope. The cracks have been observed since 2020, and no significant changes were noticed. No other permafrost degradation was observed other than the cracks noted here.

In general, SP1 is performing adequately with no remediation required.

6.2 Saline Pond 3

SP3 was constructed during the winter of 2018/2019 in the south portion of the P3 area for the temporary storage of saline water from underground operations. It is an HPDE-lined pond with a storage capacity of 5,000 m^3.

The pond is surrounded by perimeter berms constructed with rockfill. A layer of bedding material was placed over the native ground and rockfill berms. A HDPE geomembrane liner was placed over the base of the perimeter berms.

The inspection involved walking along the crest of the saline pond perimeter berm, to examine the berm for visual signs of deformation and instability, cracking, uneven surfaces, and seepage. A selection of photographs from the inspection is included in Appendix I. The photo locations are presented in Figure 10.

At the time of the inspection of SP3, the following general observations were made:

- SP3 was near empty at the time of the inspection (Photos 6 and 8, Appendix H).
- The perimeter berms were in good condition with no significant signs of cracking or settlement (Photos 7, 9, and 10, Appendix I).
- A small amount of erosion has occurred along the crest of the berms, but it does not impact the performance of the pond.

- The HPDE liner above the water level appeared to be in good condition. It is understood that mine personnel regularly perform a liner inspection when the pond is drained.
- No seepage from the pond was observed.

Overall, SP3 appears to be performing adequately.

7.0 DIVERSION CHANNELS AND BERMS

7.1 Background

This section covers the inspection of diversion Channels 1, 2, 5, 7, 8, Berm1, Berm3, and Channel2 Berm. The inspection of diversion Channels 3, 4, 9, 10, 11, Berm2, and Berm4 were covered in Section 5.0. The selected photos from the inspection are included in Appendix J. The photo locations are presented in Figure 11.

The channels were constructed by excavating a trench, placing rockfill, placing non-woven geotextile to line the rockfill/excavation, and then placing riprap (coarser rocks) over the fabric to line the channels. The berms were constructed by using a combination of esker material and till.

Channel1 is designed to move water from former Pond H13 to CP1 and extends from Culvert2 to Pond H9 along the north and east sides of Portal No. 2. Channel1 is approximately 493 m long with a base width of approximately 3 m.

Channel2 is located along the northern end of the main mine site industrial pad and is approximately 270 m long with a base width of 1 m. During construction and operation, contact water from the area is expected to flow into Channel2, which in turn eventually flows into CP1.

Channel5 and Berm3 are located west of CP5 and are designed to divert water from the Pond A12 catchment area into CP5 so that this water does not flow into the downstream TIRI#1 open pit. Channel5 is the main water diversion structure; Berm3 is required to temporarily retain water under an extreme rainfall event when the water level in CP5 is high. Channel5 is approximately 429 m long with a base width of approximately 3 m. Berm3 is approximately 315 m long with a maximum height of about 2.8 m. Berm3 consists of a till core approximately 2 m wide, a foundation key trench backfilled with till, and a cover layer constructed out of 600 mm minus esker material.

Channel7 is a water collection channel that collects flow from Culvert11 and part of the runoff from the laydown area and directs the water to Channel1.

Channel8 is a water collection channel located on the west side of Portal No. 2 to collect part of the surface flow of WRSF1 and facilitates flow of site drainage through Culvert2 and Channel1.

Berm1 is required to protect Portal No. 2 from flooding during extreme rainfall events when potential ponding occurs in the area.

Channel 2 Berm was constructed in 2023 to prevent potential outflow from Channel 2 from entering Lake G2. The berm was predominantly constructed of esker material, with rockfill material covering it.

7.2 Visual Observations

Channel1

The inspection of Channel1 involved walking along the channel from Culvert 2, around the crusher ramp. The water level in the eastern portion of the channel is controlled by that of Pond H9. Channel1 is shown in Photos 1 through 5, in Appendix J. Thaw subsidence at various areas along the lower reach portion of the channel and ponded water were observed (Photos 4 and 5, Appendix J) during the 2025 annual geotechnical inspection.

Cracking and settlement were observed along the edges of the channel. This was likely caused by thaw subsidence and thermal disturbance from runoff water flowing into Channel1.

Channel2

Channel2 was inspected by walking from the channel outlet culvert towards the top of the channel behind the accommodations complex. As noted in previous years, the slope of the channel base is not consistent, and some pooling of water and deposition of sediment occurs in lower areas. No geotechnical concerns associated with Channel2 were identified. Channel2 is shown in Photos 6 through 8, Appendix J.

Channel2 is intended to drain into a low wet area that drains through Culvert13, which eventually drains south towards Channel1 and CP1. The conditions of Channel2 are like those observed in 2024, and ponding water at various lower areas does not affect the channel performance.

Channel5

Channel5 was inspected by walking along its length. Channel5 is shown in Photos 9 to 12, Appendix J. Overall, the western portion of Channel5 appeared stable, with no geotechnical concerns, while the eastern portion appeared stable with some thaw subsidence observed (Photos 9, 10, and 12, Appendix J). A slumping area previously observed in 2023, was remediated during the reconstruction of the western portion of Channel5. The riprap placed along the channel slopes in the region of the former pond has subsided below the elevation of the ponded water within the channel. Water was ponded within the portions of the channel. The upper reach of the channel was filled with minor sediments from erosion; cleanup action is not required at this stage.

Channel7

Channel7 was inspected by walking along its length. The channel is shown in Photos 13 and 14, Appendix J. There is ponded water in portions of the channel due to some subsidence in the channel base. The conditions at Channel 7 are similar to those observed during the 2024 annual geotechnical inspection.

Channel8

Channel8 was inspected by walking along portions of its length. No geotechnical concerns were identified along the channel. The conditions of Channel8 are like those observed in 2024.

Berm1

Berm1 was inspected by walking along its length. A 350 mm diameter culvert has been installed in the channel for access to the laydown area adjacent to Portal No. 1. No geotechnical concerns were identified along the Berm.

Berm3

Berm3, adjacent to Channel5, was inspected by walking along the crest and slopes and examining the condition of the berm for visual signs of deformation and instability, cracking, or uneven surfaces. A selection of photographs from the inspection are included in Appendix J (Photos 15, 16, 17, and 18, Appendix J). Cracking was observed along the berm crest and side slopes (Photos 15, 16, 18, and 19, Appendix J), the cracks have grown in width and depth compared to observations made during the 2024 annual geotechnical inspection. Settlement and rutting from equipment traffic was observed at the west abutment of Berm3 that was approximately 0.25 m deep on the berm top surface, as shown in Photo 19, Appendix J. The settlement does not impact the functionality of the berm. A layer of rockfill was placed between Berm3 and Channel5 in 2024 to provide positive drainage toward Channel5. It is recommended that the crest and side slopes of Berm3 be repaired to remediate the erosion and cracking observed during the 2025 annual geotechnical inspection.

Channel2 Berm

Channel2 Berm was inspected by walking along the crest and slopes and examining the berm for visual signs of deformation, instability, cracking, or uneven surfaces. Channel2 Berm is in good condition with no geotechnical concerns identified along the berm (Photo 19, Appendix J).

7.3 Summary and Recommendations

The following recommendations are provided regarding the diversion channels and berms:

- Continue to monitor subsidence at the base of Channel2 to determine if it impacts the channels performance.
- Tetra Tech was informed by Agnico Eagle that maintenance and repair at Channel7 were fully completed after the 2025 annual geotechnical inspection. Tetra Tech will confirm the maintenance and repairs at Channel7 during the 2026 annual geotechnical inspection. Maintenance at Channel1 was mostly completed, except for a small section in the lower reach, and Tetra Tech will also confirm the maintenance work conducted at Channel1 during the 2026 annual geotechnical inspection.
- Repairs and maintenance should be performed on Channel1 and Channel7 to promote drainage.
- The western portion of Channel5 was reconstructed in 2024 and is performing adequately. It is recommended that the eastern portion of Channel5 be repaired and maintained.
- It is recommended that Berm3 be repaired to remediate the erosion and cracking was observed along the crest and side slopes of Berm3 during the 2025 annual geotechnical inspection.

8.0 TAILINGS STORAGE FACILITY

8.1 Background

A filtered TSF is being constructed at the Mine. Water is pressed out of the tailings in the process plant. The tailings are temporarily stored in the Tailings Dewatering Building next to the process plant, known as the “Church”; from there, they are loaded onto trucks and hauled to the TSF.

The tailings are dumped in the TSF, spread with a dozer in 0.3 m lifts with survey control, and compacted. The tailings are progressively reclaimed by placing a rockfill cover on the exterior slopes as the tailings stack rises.

During the inspection, Cell 1 was constructed to its final design and Cell 2 was being used for active tailings placement.

8.2 Visual Observations

In general, the TSF is operated in accordance with the TSF OMS manual (Agnico Eagle 2025). The tailings are dumped in the TSF, spread in 0.3 m lifts and compacted (Photos 6, 7, and 8, in Appendix K). At the time of the inspection, Cell 1 of the TSF was constructed to approximately 100 m masl (without closure cover placement) and Cell 2 to approximately 76 masl at the eastern portion and 90 masl at the western portion. Selected photos from the inspection are included in Appendix K. The photo locations are presented in Figure 12.

Surface erosion was previously observed in various locations on the north slope of Cell 1 and Cell 2. These conditions have not been observed since the 2023 inspection and were likely due to precipitation events leading up to the 2023 annual geotechnical inspection. A rockfill berm was previously constructed at the Cell 1 and Cell 2 tie-in location to reduce surface erosion, and it has since been covered with tailings from placement in Cell 2. To prevent erosion, rockfill cover was placed along the north slope and crest of Cell 1.

In 2024, the crest of the rockfill slope against Cell 1 experienced minor cracking and settlement, which was not observed during the 2025 annual geotechnical inspection. The rockfill slope cover around the tailings appeared stable and was performing adequately (Photos 1 to 4, in Appendix K).

During the 2025 annual geotechnical inspection, an area of ponding water was observed along the east toe of the TSF (Photo 1, in Appendix K).

8.3 Instrumentation and Monitoring

Agnico Eagle's geotechnical engineers prepare weekly inspections and monthly analytical reports describing the tailings placement and design verification updates. The tailings have an optimum moisture content of 15.4% and are typically placed at moisture contents ranging from 12.3% to 20.8% with an average of 17.0%. The measured porewater salinity of the tailings between October 2024 and September 2025 ranges from 12.0 parts per thousand (ppt) to 15.8 ppt with an average of 13.0 ppt, which is lower than the assumed 15 ppt for the design. Additional testing includes ARD/ML sampling and testing, process water analysis, including salinity testing, and quarterly off-site geotechnical verification (moisture-density testing and particle size analyses).

GTCs are installed at eight locations within the placed tailings. The measured ground temperatures are presented in Appendix K. GTC-01A and GTC-02 are now located within the rockfill covered slope of the TSF.

Measurements taken between October 2024 and October 2025 indicate that the foundation in Cell 1 had an average ground temperature of -3.6°C and was relatively stable compared with measurements taken prior to September 2022. The upper tailings (the top 3 m of tailings) had an average ground temperature of -1.6°C between October 25, 2024 and October 25, 2025. Freeze back is developing with time, as the tailings have reached the final design elevation and are covered with a layer of waste rock. Measurements taken between October 2024 and October 2025 indicate that the temperature of the lower tailings (deeper than 3.0 m from the top of the tailings surface) ranges from -0.5°C to -3.5°C and are generally stable throughout the year.

Measurements taken on October 25, 2025 indicate that the ground temperature of the foundation in Cell 2 ranges from -2.0°C to -5.5°C. Based on data collected from TSF-GTC06 and TSF-GTC07, the first 0.8 m thick layer of tailings placed above the original ground is in a frozen condition, while the remaining tailings above this layer are in

an unfrozen condition. At TSF-GTC08, the temperature data shows that the first 7.9 m of tailings placed above the original ground are in a frozen condition. There is no available ground temperature data for tailings above the first 7.9 m placed at TSF-GTC08. For TSF-GTC06 and TSF-GTC07 an increase of temperature ranging from 0.5°C to 1.2°C was observed within the first 2 m of tailing placed and freeze back was observed above the first 2 m of tailings placed. At TSF-GTC08, freeze back was observed within the first 4.9 m of tailing placed. Freezing back is expected with time as more tailings are placed in Cell 2.

Nuclear density tests on the in situ placed and compacted tailings performed from September 2024 to September 2025 indicate that the density of the filtered tailings meets or exceeds the design target of 92% of the maximum dry density obtained from Standard Proctor tests (i.e., 1,785 kg/m^3), for the most part, that the density of the filtered tailings is generally higher than 95% of the maximum dry density. The typical nuclear density test ranges from 1,701 kg/m^3 to 1,854 kg/m^3 with an average of 1,788 kg/m^3. The placed tailings material show very few evidence of bleed water and are easily trafficable after placement and compaction.

8.4 Water Management

WMIs for the TSF are operated as per the Meliadine Mine water management protocols. Approximately half of runoff naturally drains or is diverted via Channel3 to CP3, and the remaining half reports to CP1 via various culverts and Channel1.

8.5 Summary and Recommendations

The TSF appeared to be functioning well at the time of the inspection. No geotechnical concerns were identified, and no signs of permafrost degradation were observed in the TSF area.

The TSF perimeter rockfill cover appears to be functioning well from a geotechnical perspective. Minor cracking was observed on the crest of the rockfill cover at Cell 1 during the 2024 annual geotechnical inspection. However, this was not observed in the 2025 annual geotechnical inspection. The rockfill crest should continue to be monitored for cracking or degradation.

It is recommended that regrading of the rockfill slope be completed at the southwest corner of the TSF. The north slope of Cell 2 should be maintained at a minimum slope of 4 Horizontal to 1 Vertical if no rockfill cover is placed during the construction. Consideration should be given to backfill and grade the area of ponded water at the east toe of the TSF.

9.0 WASTE ROCK STORAGE FACILITIES

Waste Rock Storage Facilities WRSF1, WRSF3, and WRSF6 are used to dispose of waste rock and overburden from the Pump open pits, Tiriganiaq open pits, and the underground operations. The waste rock and till are stored in separate areas within each facility. The design drawings for WRSF1, WRSF3, and WRSF6 and photos are included in Appendices L, M, and N, respectively. The photo locations are shown in Figures 13, 14, and 15. Observations of each facility are noted below.

9.1 WRSF1

Disposal in WRSF1 began in 2019, with most of the material being placed since December 2020. Benches 77, 82, 87, 92, 94.5, 97, 102, and 107 m, bench had been placed to the design limits at the time of the 2025 annual

geotechnical inspection. A portion of the 112 m bench was filled with waste rock in 2025, and another portion of the 112 m bench was used for the temporary storage of mineralized waste. As per the design, till was placed in the centre of the facility with a 40 m perimeter of waste rock around the till. Most of the till was placed in the winter.

The till placed in WRSF1 is a combination of material placed prior to the summer of 2019, and that placed during the winter of 2019/2020 and 2020/2021. The winter placed till was wet; it is speculated that it contained some ice rich material and is thawing and consolidating over the summer. As of September 2025, approximately 2.2 Mm^3 of till and 3.2 Mm^3 of waste rock have been placed in WRSF1, which is about 96% of the total design storage capacity. No overburden material was placed at WRSF1 in 2025 (January to September 2025). Some mineralized waste was placed in WRSF1 in September 2025 and periodically for the rest of 2025. Ground temperatures at the base of the WRSF1 facility are being monitored with vertical and horizontal GTCs. The cable locations are shown on the design drawings. The measured ground temperatures are presented in Appendix L. Based on these measurements the foundation of the waste rock pile is frozen. Horizontal beads roughly 70 m inside from the toe of the pile have warmed by 0.2°C to 0.3°C between November 25, 2024 and November 25, 2025. The temperatures within the foundation appear to be stable and remain well below zero (-6.0°C). From the data collected on October 25, 2025, the thickness of the active layer within the waste rock layer ranged from 2.5 m to 4.6 m.

The material has generally been placed in the pile according to the WRSF1 design. No geotechnical issues were observed during the 2025 annual geotechnical inspection.

9.2 WRSF3

Disposal in WRSF3 began in 2020. The overburden was placed to the 77 m bench, and waste rock was placed to Bench 87 m, with ongoing construction at Bench 92 m observed at the time of the annual inspection. The till placed in WRSF3 appeared to be well compacted due to dozer compaction. Settlement and cracking previously observed during the 2023 annual geotechnical inspection on the east side of the 72 m bench of the waste rock (adjacent to CP2 area) were mitigated by placing additional waste rock and traffic compaction. No settlement and cracking were observed in this area during the 2025 annual geotechnical inspection.

Similar to conditions observed in 2023 and 2024, water was ponding at the southwest corner of WRSF3. If no mitigation measures are taken, the ponded water could cause permafrost degradation in that area. The current mitigation control to manage the ponded water adopted by Agnico Eagle is to pump the water out of this area as required, with plans to construct a ditch and sump at the southwest corner of WRSF3.

As of September 2025, approximately 3.1 Mm^3 of till and 5.6 Mm^3 of waste rock have been placed at WRSF3. No overburden material was placed in WRSF3 in 2025 (January to September 2025), and about 1.1 Mm^3 of waste rock was placed in WRSF3 in 2025 (January to September 2025). The 87 m bench at the southeastern part of WRSF3 did not reach the design limit at the time of the 2025 annual geotechnical inspection. Tetra Tech was informed by Agnico Eagle that the area was filled to the design limits in December 2025. Tetra Tech will confirm this work during the 2026 annual geotechnical inspection.

Ground temperatures at the base of the WRSF3 facility are being monitored with vertical and horizontal GTCs. The cable locations are shown on the design drawings. The measured ground temperatures are presented in Appendix M. HGTC-02 within the WRSF3 foundation stopped taking measurements on July 26, 2022. It was determined that the GTC was damaged and cannot be repaired. The remaining GTCs are sufficient to monitor the WRSF3's performance at this time. Based on measured ground temperatures at other GTC locations, the foundation of the waste rock pile is frozen. The average temperatures within the foundation at VGTC-01 warmed from -3.7°C in November 2024 to -3.2°C in November 2025. The average temperatures within the foundation at VGTC-03 have

warmed by 0.3°C between November 2024 and November 2025. No permafrost degradation was observed around WRSF3 at the time of the inspection.

In general, WRSF3 is performing well with no geotechnical issues noted during the inspection. The following recommendation for improvement was made based on the inspection:

- Water should be pumped out regularly before the construction of the ditch and sump planned for the southwest corner of WRSF3. The ditch and sump will help to divert/collect runoff water from WRSF3, prevent potential permafrost degradation at the WRSF3 toe, and reduce the efforts required for regular pumping during operation.

9.3 WRSF6

Disposal in WRSF6 began in 2025. The overburden was placed to Bench 74 m, and waste rock was constructed to Bench 74 with ongoing construction for Bench 79 m at the time of the annual inspection. As of September 2025, approximately 0.42 Mm^3 of till and 0.28 Mm^3 of waste rock have been placed in WRSF6. Overburden material was placed between January and May 2025, and waste rock placement commenced in March 2025.

Ground temperatures at the base of the WRSF6 facility are being monitored with vertical and horizontal GTCs. The cable locations are shown on the design drawings. The measured ground temperatures are presented in Appendix N. Based on the ground temperatures at the GTC locations; the foundation of the waste rock pile is frozen. The temperatures within the foundation range from -1.6°C to -6.8°C with an average temperature of -4.0°C on November 25, 2025. No permafrost degradation was observed around WRSF6 at the time of the inspection.

Overall WRSF6 is performing well with no geotechnical issues noted during the inspection. Placement of overburden and waste rock were generally per the placement protocols and consistent with design.

10.0 SITE ROADS

10.1 Background

The site has numerous roads, including haul roads, service roads, roads to borrow areas, and other facilities. The following is a list of roads inspected. Photographs of the site roads are included in Appendix O. The photo locations are presented in Figure 16.

- TSF and landfill access road;
- Main site pad area roads;
- Main site water intake access road;
- Emulsion plant pad access road;
- Tiriganiaq Esker access road;
- Magazine storage area and access road;
- Wesmeg access road, Wesmeg esker area, and vent raise;
- CP3 access road; and

- CP4 access road.

10.2 Visual Observations

At the time of the inspection, the site roads were generally in good condition. Select photos of the roads are included in Appendix O. The roads appeared to be of adequate width with pull-outs where required to allow vehicles to safely pass. The heights of the road fills were such that berms were not required. Many of the roads appeared to have been constructed using a combination of sand and gravel obtained from esker borrow areas, rockfill, and crushed aggregate.

The road surfaces get muddy when wet. The roads are graded regularly. No geotechnical concerns were identified during the inspection. No permafrost degradation was observed along the road at the time of the inspection.

Permanent water management culverts are in place through road fills. Culverts observed were: Culverts 1, 2, 3, 4, 7, 8, 10, 11, 13, 15, 16, 18, and 20. The culverts were generally in good condition with the exception of Culvert 18, through the TSF road, which has been crushed to half its original height (Photos 13 and 14, Appendix O).

10.3 Summary and Recommendations

The site mine roads and culverts were generally well maintained and in good geotechnical condition at the time of the inspection. No specific recommendations for geotechnical improvements are provided. It is anticipated that normal maintenance of the roads will be conducted as required.

11.0 BORROW SOURCES

11.1 Background

Numerous borrow sources were developed during the construction of the Meliadine Mine, and many of them were reclaimed in 2019. The following borrow areas were observed:

- Meliadine North Esker;
- Meliadine Esker; and
- Wesmeg Esker.

Photographs of the borrow areas are in Appendix P. The photo locations are presented in Figure 17.

11.2 Visual Observations

In general, the borrow areas were in good condition and had been reclaimed by grading to knock down various piles and ruts. No permafrost degradation was observed around the borrow sources at the time of the inspection.

11.3 Summary and Recommendations

The borrow areas should be monitored for future erosion and thaw settlement; however, they appear to be performing well since they were reclaimed three years ago.

12.0 ORE STOCKPILES

12.1 Background

The ore storage areas are located east of the crusher area. Photos of the ore stockpiles are included in Appendix Q. The photo locations are presented in Figure 17.

The pile heights should be constructed such that they are less than 2 m above the reach height of the loader removing material from the pile. The dig face should be carried out in a manner such that the slope angles are flatter than the angle of repose of the material (1.3H:1V to 1.4H:1V).

It is Meliadine policy that a maximum 7 m high bench face is to be used. A second bench can be constructed to a maximum total height of 12 m, with a 5 m offset from the first bench. In general, most of the piles in the ore storage area are less than 7 m. The main ore pile was placed in two benches which appeared to meet the site specifications.

The piles appeared to be stable and well managed with no signs of instability. No geotechnical concerns related to the stability of the stockpiles were identified at the time of the inspection.

13.0 OTHER MELIADINE FACILITIES

13.1 Crusher Ramp

The crusher ramp is an earth fill structure consisting of a ramp, turn around area, and loading area adjacent to the crusher. It was constructed in 2018 and mainly constructed of rockfill with gabion retaining walls surrounding the crusher. The crusher ramp is shown in Photos 1 through 4, Appendix R. The photo locations are presented in Figure 19.

The area was visually inspected. The gabion wall appears to be performing well with no visual signs of distress. It is leaning in towards the fill materials as intended.

The fill slopes were relatively smooth with no obvious cracking, erosion, or signs of instability. There was also no cracking on the surface of the ramp, turn around area, or the loading area adjacent to the crusher.

It appears to be performing well from a geotechnical perspective.

13.2 Saline Water Treatment Plant

The Saline Water Treatment Plant (SWTP) was constructed to treat water from underground operations. It was constructed in an existing storage warehouse/shop that was extended on one end. The structure is a fabricated building founded on a concrete slab.

The SWTP generates considerable heat, making the building's interior warm. The concrete slabs of both the original building and the extension have undergone considerable settlement. It is speculated that the settlement is due to the thawing of ice-rich permafrost underneath the building. The settlement was reported to be up to 0.4 m in 2019.

The facility has not been used since March 2020, and there are no plans to resume operations. The inside of the facility has not been inspected since 2020. If the facility is operated again, it is recommended that an assessment of the geotechnical and structural conditions be carried out.

13.3 Landfill

The main landfill for the Meliadine Mine is located at the northeast corner of WRSF1. The landfill has perimeter berms constructed of esker material. The landfill is used for dry waste only. Kitchen and other burnable wastes are burned in the onsite incinerator. The landfill is shown in Photos 5 through 8, Appendix R. The photo locations are presented in Figure 13.

The perimeter berms are performing well from a geotechnical perspective with no signs of instability. It is understood that the berms were raised approximately 2.0 m in 2023 to provide additional capacity in the landfill. Minor cracking and settlement are observed at the crest of the berms (Photo 7, Appendix R), it is not expected to impact the performance of the berms.

At the time of the inspection the landfill debris was predominately covered with run-of-mine rock. The landfill appeared to contain construction waste and wood not suitable for burning (painted, treated etc.) among other things.

Overall, the landfill is operated as per management plan, and the berm is performing adequately.

13.4 Emulsion Plant Pad

The emulsion plant is located at the north end of the Meliadine Mine. The plant was constructed on a pad constructed of esker material. The emulsion plant pad is shown in Photos 9 through 12, Appendix R. The photo locations are presented in Figure 19.

It is understood that the pad settled after construction, but there have been no recent reports of settlement issues. The north edge of the pad is experiencing erosion; the erosion channels are like those observed from 2019 to 2024 and are not currently impacting the use of the pad. No permafrost degradation was observed around the emulsion plant pad at the time of the inspection.

It is recommended that the pad settlement and erosion continue to be monitored. Remedial action was not required at the time of the inspection.

The storage pad next to the emulsion pad is filled with shipping containers. Several shipping containers located on the south corner of the pad are at the edge of the pad. It is recommended to position the shipping containers back from the pad's crest.

Some localized areas of depression were observed at the emulsion plant pad and its associated structures. These localized areas are not currently impacting the use of the emulsion plant pad or its associated structures and does not currently pose geotechnical risk.

13.5 Landfarm

A lined landfarm was constructed southeast of the process plant. Windrows of soil 1.0 m to 1.2 m have been placed in the landfarm as shown in Photos 13 to 17, Appendix R. The photo locations are presented in Figure 19.

The landfarm berms appear to be in a stable condition with minor cracks on the berm crest. A small amount of geomembrane liner and geotextile was exposed on the perimeter of the berm (Photo 14, Appendix R). The exposed liner will not impact the landfarm performance.

The landfarm sump contained a small volume of water at the time of the inspection (Photo 13, Appendix R). It is understood that this water is tested before being pumped out.

No geotechnical issues were noted during the inspection.

13.6 Industrial Fuel Tank Farm

The Industrial Fuel Tank Farm is located east of the process plant, as shown in Photos 18 to 22, in Appendix R. The photo locations are presented in Figure 19.

Two tanks are in the facility. The facility is lined with a geomembrane liner for secondary containment.

The crest of the berm has several cracks up to 150 mm wide; the length, depth and severity of cracks have increased from what was observed during the 2024 annual geotechnical inspection. The berms' crest should be repaired to remediate cracking. A small amount of erosion has occurred on the tank pedestals; however, it does not appear to extend under the tank bases. The cover fill over the geotextile is missing in a small area (<0.5 m diameter).

Similar to conditions observed from 2022 to 2024, crush material underneath the pipeline cribbing going over the containment berm has been eroded away. Crush material should be placed back around the pipeline supports to remove stress on the pipeline.

Overall, the tank farm is performing well from a geotechnical perspective. No permafrost degradation was observed around the facility at the time of the inspection. The repair work for the cracking on the crest of the berm was discussed between Agnico Eagle and Tetra Tech during the 2025 annual geotechnical inspection. It is understood that the repair work was completed by Agnico Eagle in December 2025. Tetra Tech will confirm this repair work during the 2026 annual geotechnical inspection.

13.7 Portal No. 1 and Portal No. 2

Portal No. 1 and Portal No. 2 provide access to the underground mining operations at the Meliadine Mine. Photos of Portal No. 1 are shown in Photos 23 to 26, Appendix R, while photos of Portal No. 2 are shown in Photos 27 to 30, Appendix R.

Overall, Portal No. 1 is performing well with no geotechnical issues noted during the inspection. Portal No. 2 is performing adequately, however when compared to the 2024 annual geotechnical inspection, increased erosion and degradation was observed underneath the Portal No. 2 strip footings which support the corrugated steel (Photos 29 and 30, Appendix R).

It is recommended that the voids underneath the footing foundations are backfilled, and erosion protection measures are put in place to prevent additional erosion along the base of the footing. Further erosion along the base of the footing may trigger the foundation instability.

13.8 Mine Site Tank Fuel Farm

The Mine Site Fuel Tank Farm is located northeast of SP1, as shown in Photos 31 to 34, in Appendix R. The photo locations are presented in Figure 19.

Two tanks are in the facility. The facility is lined with a geomembrane liner for secondary containment.

Exposed geotextile and liner were observed on the downstream slope of the southwest perimeter berm. It is recommended that additional liner cover material be placed at the exposed liner area, and the downstream slope is regraded to the design grade, the exposed liner could be cut or left in place prior to the placement of the additional liner cover material. The additional liner cover material will help to mitigate the risk of the exposed liner, including further translation slide of the cover material.

Overall, the fuel farm is performing well from a geotechnical perspective. No permafrost degradation was observed around the facility at the time of the inspection.

13.9 Pond P3

Pond P3 consists of the P3 laydown pad area, Pond P3, DP3-A (downstream berm), and an upstream ditch located northeast of P3 Laydown Pad, as shown in Photos 35 to 42, in Appendix R. The photo locations are presented in Figure 19.

Overall, Pond P3 and its associated structures were performing adequately at the time of the inspection. The following was observed:

- No geotechnical concerns were identified for Pond P3 or the P3 laydown pad area; both structures are performing well.
- Localized ponding was observed in the upstream ditch (Photo 37, Appendix R), this ponding is likely due to thaw subsidence and settlement of the existing ground. It is not expected to affect the performance of the ditch.
- Cracking was observed along the crest of berm DP3-A (Photos 41 and 42, Appendix R). The cracking observed is from thaw subsidence beneath the berm. It is recommended that the cracking observed on the top of the berm be monitored and assessed, and that a mitigation plan be developed if required. The crest of the berm should be monitored for future cracking and settlement.

13.10 Other Facilities

The following facilities were inspected during the 2025 annual geotechnical inspection:

- New Cyanide Storage Pad, constructed in 2019;
- Emulsion Plant Storage;
- Freshwater Intake;

- Incinerator Pad;
- Paste Plant Ramp; and
- Industrial Pad;

No geotechnical issues were noted in these facilities. No permafrost degradation was observed around these facilities during the inspection.

14.0 EXPLORATION CAMP AND ACCESS ROAD

Portions of the exploration camp had been dismantled at the time of the 2025 annual geotechnical inspection. Some of the dorms had been removed from the area, although other portions of the camp were still in use. Appendix S contains photographs taken during the inspection. The photo locations are presented in Figure 18.

The freshwater inlet for the exploration camp appears to be out of use. The station support beams appear to be eroded away at one corner (Photo 4, Appendix S). The beam should be repositioned for stability.

The landfarm at the exploration camp access road is in the process of decommissioning.

The access road to the exploration camp was in good condition. There are several depressions in the road down to the diffuser at the east end of the exploration camp area, with no concerns associated with these depressions.

15.0 ALL-WEATHER ACCESS ROAD AND ASSOCIATED WATER MANAGEMENT STRUCTURES

15.1 AWAR Road

The AWAR construction activities began in the winter of 2012 and were completed by the end of October 2013 to connect the hamlet of Rankin Inlet to the Meliadine Mine. Appendix T contains photographs taken during the inspection. The road is approximately 23.8 km long, with three bridge crossings and culverts installed at 26 locations. The road was designed to be 6.5 m wide for most of its length with pull outs approximately 400 m +/-50 m to allow two-way traffic. This keeps the AWAR and By-pass road traffic consistent.

The AWAR is used by Agnico Eagle and provides unrestricted all-terrain vehicle (ATV) access for the public, if it is safe to do so. The AWAR is used to transport building materials, construction/mining equipment, fuel, reagents, supplies, workers, and contractors to the Meliadine Mine.

The road design is based on a general sub-base composed of rockfill or sand and gravel from esker sources and crushed granular surfacing with a combined minimum thickness of 500 mm. The road design varied based on the relative susceptibility to freeze and thaw induced settlement of the foundation soils. The thickness of the road fill material was generally increased, to a minimum of 1.3 m, in areas where potentially thaw-sensitive soils were identified. Along portions of the road where thaw-sensitive soils were identified, a geotextile material was incorporated into the road design to limit damage to the road should the foundation material thaw.

15.1.1 Observations and Recommendations

The road and culverts were generally in good condition at the time of the inspection, with the exceptions noted below. Most culverts were unobstructed with no signs of substantial damage to the culverts. All bridges and their embankments were in good geotechnical condition at the time of the inspection. A structural and/or mechanical assessment of the bridges was not conducted and is beyond the scope of this geotechnical inspection.

The locations and a photographic record of the inspected culverts and bridges are provided in Appendix T. The photo locations are shown in Figures 25 to 34.

Table 15-1 lists the locations of water management structures: culverts and bridges that have been installed along the AWAR. The location of the culverts and bridges are listed, based on distance from the Healing Centre in Rankin Inlet, with the gate house at KM12. Size and number of culverts are provided in Table 15-1, along with specific observations and photos at the time of the inspection, and any recommendations.

It is understood that Agnico Eagle has implemented a watercourse crossing inspection and maintenance program, which includes:

- A regular inspection program to identify issues relating to watercourse crossings, such as structural integrity and hydraulic function;
- An event-based inspection program to track the impacts of larger storm events on watercourse crossings; and
- Observations to confirm water is flowing through the culverts, and no sediment is being transported in the water, to determine if any mitigation is required.

Road maintenance and snow management are carried out, as deemed necessary. Steaming of culverts is included as a maintenance activity. Agnico Eagle adds additional crushed aggregate on the AWAR annually and applies calcium chloride for dust control through the summer.

The construction of a waterline along the AWAR between the Meliadine Mine and the ocean near Rankin Inlet was ongoing at the time of the inspection. The construction of the waterline resulted in the damage or burial of pipe sleeves along the AWAR.

In general, the road appeared to be in good geotechnical condition at the time of the inspection. No obvious permafrost degradation was observed along the road during the inspection. Recommendations for improvements to the water management structures are presented in Table 15-1. There are numerous locations where there are no culverts or where the pipe sleeves were installed to facilitate the water pumping during the freshet. Water ponding against the AWAR or poor drainage was observed at these locations. Tetra Tech recommends monitoring drainage conditions during the freshet period and after heavy rainfall events. If monitoring indicates that a culvert is required to improve the drainage, a mitigation plan should be developed and implemented. Several additional culverts received damage to their inlets and outlets likely during snow clearing activities or the waterline construction and are summarized in Table 15-1 with associated photos.

During the 2025 annual geotechnical inspection tetra Tech observed that culverts were installed at three additional locations (KM 4.1, KM 4.3, and KM 8.7), and a culvert upgrade was performed at KM 16.3. The details and conditions for these culverts are presented in Table 15-1.

It was noticed that some culverts do not have signage to facilitate easy identification, some culverts were mis-labeled, and some KM stations along the AWAR appeared to be out of alignment. It is recommended that the signage for culverts and KM stations be reviewed for accuracy and updated where required.

Table 15-1: AWAR Road – Water Management Structures Summary

Station (distance from Friendship Centre)	Water Management Structure Description	Conditions, Observations, and Recommendations (at time of inspection)	Photo Page
KM 3.9	1 x 160 mm steel pipe, used as culvert	Good condition – no flow at the time of inspection, stable embankments.	AWAR Road Culvert – Photos 1 and 2
KM 4.1	1 CSP culvert	Water was ponding at the culvert inlet due to a partial blockage from eroded material. The culvert outlet is damaged but in good condition overall. Minor flow was observed during the time of the inspection. **Recommendation: Clear the culvert inlet.**	AWAR Road Culvert – Photos 3 and 4
KM 4.8	1 x 160 mm steel pipe, used as culvert	Good condition – no flow at the time of inspection, stable embankments.	AWAR Road Culvert – Photos 5 and 6
KM 5.4	1 x 600 mm CSP	Good condition – located in the community portion of the road. Minor ponding water at the inlet and outlet at time of inspection. Damage to culvert at the inlet and outlet and an erosion pit at the outlet, no armouring is present at the outlet.	AWAR Road Culvert – Photos 7 and 8
KM 6.0	Char River Bridge	Good condition – stable embankments and abutments are armoured. Similar conditions as observed during the 2024 annual geotechnical inspection.	AWAR Road Culvert – Photos 9 to 11
KM 6.1	3 CSP culverts: 2 x 1,300 mm 1 x 700 mm	The culverts are vertically offset with the 700 mm culvert elevated above the 1,300 mm culverts. Some minor erosion observed between the culverts on the downstream side. All clear and in good condition. Minor ponding water in the lower 1,300 mm culvert, upstream of the road, and downstream of the road. Armouring appears to be adequate. Small Crack in 700 mm outlet. East side 1,300 mm culvert has deflection under the road. There is little change to the cross-sectional area.	AWAR Road Culvert – Photos 12 and 13
KM 6.9	3 CSP culverts: 2 x 1,000 mm 1 x 700 mm	The culverts are vertically offset with the 700 mm culvert elevated above the 1,000 mm culverts. The 700 mm culvert had a dent inside. West 1,000 mm culvert contains dent at the bottom inlet. East 1,000 mm culvert has minor erosion at outlet with no armouring. Ponded water observed in the lower culvert. Small amount of water ponded downstream. Sandy soil around culverts, potential for erosion, but none noted during inspection.	AWAR Road Culvert – Photos 14 and 15
KM 7.0	3 CSP culverts: 2 x 1,000 mm 1 x 700 mm	Vertically offset. 700 mm culvert is elevated. Water ponded in the southern 1,000 mm culvert, minor deformation of culverts under the road, no substantial reduction of cross-sectional area. The culverts and riprap appear in good condition. Low flow at time of inspection.	AWAR Road Culvert – Photos 16 and 17

Table 15-1: AWAR Road – Water Management Structures Summary

Station (distance from Friendship Centre)	Water Management Structure Description	Conditions, Observations, and Recommendations (at time of inspection)	Photo Page
KM 7.35	3 CSP culverts: 1 x 900 mm 1 x 700 mm 1 x 1,000 mm	Vertically offset. 700 mm culvert is elevated. Significant damage to the inlet and outlet of the 900 mm culvert, and the inlet of the 1,000 mm inlet. Minor damage to the inlet and outlet of the 700 mm culvert and the outlet of the 1,000 mm culvert. Erosion potential due to finer grained soils around 700 mm culvert at the inlet and outlet, but no significant erosion noted. 1,000 mm culvert clear. Minor ponding water at inlet of 900 mm culvert. Agnico Eagle indicates culverts performed well during 2021/2022 freshet. All culvert outlets are damaged, likely due to the snow removal process prior to the freshet season. **Recommendation: Repair culvert damage.**	AWAR Road Culvert – Photos 18 and 19
KM 8.0	Meliadine River Bridge	West abutment, slopes upstream and downstream of bridge have exposed sand and gravel; no erosion noted. No geotechnical concern at the time of the inspection.	AWAR Road Culvert – Photos 20 to 23
KM 8.7	1x150 mm HDPE Pipe, used as pipe sleeve.	Good condition – no flow at the time of inspection, stable embankments.	AWAR Road Culvert – Photos 24 and 25
KM 8.8	2 x 150 mm HDPE pipes, used as pipe sleeve	Agnico Eagle reported that there was water overflow in the 2023 freshet. Two HDPE pipes were installed to act as a pipe sleeve to pass a hose through for freshet pumping activities. Minimal armouring at pipe outlets. The construction of the waterline has blocked the pipe sleeve outlets, and it appears that the pipe sleeve installed at KM 8.7 will function as the replacement. It is observed that the road in this location has a low profile.	AWAR Road Culvert – Photos 26 and 27
KM 9.3	2 x 1,000 mm CSP culverts	Minor deformation of both culverts under the road. No ponded water. Armoured, no obvious signs of erosion. The road has been raised since the 2020 inspection.	AWAR Road Culvert – Photos 28 and 29
KM 9.6	1 x 1,300 mm CSP culvert	Water ponded on upstream side of culvert/road with no low flow due to elevated inlet of CSP. CSP in good condition. Erosion pits at the outlet with no armouring present.	AWAR Road Culvert – Photos 30 and 31
KM 10.5	M-5 Bridge	Good condition, stable embankment, and abutments of the bridge. Exposed geotextile at base of downstream end of left abutment has been covered. Gabion damaged on downstream of left (north) abutment. No obvious signs of erosion. Additional armouring was previously added to the southeast side abutment, where minor sloughing and cracking, erosion pathways, and settlement was previously observed.	AWAR Road Culvert – Photos 32 to 34
KM 12.2	4 CSP culverts: 2 x 1,300 mm 1 x 900 mm 1 x 700 mm	Vertically offset. 700 mm and 900 mm culverts are elevated. Minor small dents and bending of haunches in 700 mm, 900 mm, and one of the 1,300 mm culverts. Minor flow through the lowest of the 1,300 mm culverts. Minimal armour; however, no obvious erosion. Some crushing (oval shape) of culvert. Embankment slope is generally in good condition.	AWAR Road Culvert – Photos 35 and 36

Table 15-1: AWAR Road – Water Management Structures Summary

Station (distance from Friendship Centre)	Water Management Structure Description	Conditions, Observations, and Recommendations (at time of inspection)	Photo Page
KM 12.6	No culverts	Area of poor drainage. In good condition, ponding water is present but no signs of water flow at time of inspection. **Recommendation: Monitor the performance during freshet or after intense rainfall event to determine the requirement of a culvert.**	AWAR Road Culvert – Photos 37 and 38
KM 13.4	No Culverts	Erosion of bedding material observed along the waterline bench. **Recommendation: Repair the pipe bench with bedding material to design grade.**	AWAR Road Culvert – Photo 39
KM 13.75	5 CSP culverts: 3 x 1,300 mm 2 x 900 mm	Vertically offset, 900 mm culverts are elevated above 1,300 mm culverts. Good condition, no flow, minor dents, and deflection in haunch, otherwise in good condition.	AWAR Road Culvert – Photos 40 and 41
KM 14.5	No Culverts	Area of poor drainage. In good condition, ponding water is present but no signs of water flow at time of inspection. **Recommendation: Monitor the performance during freshet or after intense rainfall event to determine the requirement of a culvert.**	AWAR Road Culvert – Photo 42
KM 14.9	Access road to B12 quarry, 500 mm HDPE corrugated culvert	No ponding water, minor dents observed in culvert, Erosion at outlet. Culvert and embankments are generally in good condition. Damage to the culvert inlet.	AWAR Road Culvert – Photos 43 and 44
KM 15.4	No Culverts	Area of poor drainage. In good condition, ponding water is present but no signs of water flow at time of inspection. High water mark was observed along the eastern slope. **Recommendation: Monitor the performance during freshet or after intense rainfall event to determine the requirement of a culvert.**	AWAR Road Culvert – Photo 45
KM 16.0	No Culverts	Area of poor drainage. In good condition, ponding water is present but no signs of water flow at time of inspection. Erosion of the road was observed adjacent to the waterline on the east side of the road. **Recommendation: Monitor the performance during freshet or after intense rainfall event to determine the requirement of a culvert.**	AWAR Road Culvert – Photos 46 and 47

Table 15-1: AWAR Road – Water Management Structures Summary

Station (distance from Friendship Centre)	Water Management Structure Description	Conditions, Observations, and Recommendations (at time of inspection)	Photo Page
KM 16.5	2 CSP culverts: 2 x 1,200 mm	In 2025 the three CSP culverts previously install at this location were removed and replaced with two 1,200 mm diameter culverts. The embankments of the road were also regraded and armoured during the culvert replacement. The culverts are in good condition; recently placed material was blocking a portion of the culvert outlet. No flow or ponding water was observed. The area is armoured and no signs of overflow were present.	AWAR Road Culvert – Photos 48 and 49
KM 18.4	2 CSP culverts: 1 x 900 mm, 1 x 1,000 mm	Vertically offset culverts. The 900 mm culvert is elevated above the 1,000 mm culvert. Lower culvert has ponding water but no flow, minor dent on upstream end. Upper culvert is in good condition. Upper culvert is high on the embankment and has thin cover on the upstream side. Trench exists along upstream toe of road connecting the culverts at KM 18.1 to KM 18.15. Culvert appears to replace KM 18.15 culvert. No erosion noted, appears to be performing adequately.	AWAR Road Culvert – Photos 50 and 51
KM 18.5	1 x 600 mm CSP culvert	The culvert no longer appears to be useful as ponding is controlled by the KM 18.1 culvert. Inlet is damaged but still functional, outlet is placed high on the embankment.	AWAR Road Culvert – Photos 52 and 53
KM 19	No culverts	Ponding on north side of road, reportedly the water ponds here year-round and can reach as high as halfway up the embankment. Water 1 m below road at time of the 2019 and 2020 Inspections. 2018 Inspection reports by Agnico Eagle note straw logs were placed at KM 19, so there may have been some flow over the road in this area in 2018. No overflow reported in 2019. Agnico Eagle reported that there was overflow in 2020 freshet, but no pumping was required during 2021 freshet. An esker sand berm was constructed in 2024 to bury the waterline at this location which could reduce the potential of the overflow. **Recommendation: Monitor the water ponding area during freshet or after intense rainfall event to determine the requirement of a culvert to reduce the risk of overflow.**	AWAR Road Culvert – Photo 54
KM 21.0 to 21.5	No culverts	Low ponded water on west side of road near KM 20.0 and KM 21.2. Water reportedly flowed over the road near KM 21.5 during the 2017 freshet. Straw logs were placed on east side of road embankment to control suspended solids in the flow in 2018. No reports of overflow in 2019. Inspection in 2019 had a “wash zone” of road embankment indicating likely high water in 2019. Pumping was required in 2020. An esker sand berm was constructed in 2024 to bury the waterline at this location which could reduce the potential of the overflow. **Recommendation: Monitor the water ponding area during freshet or after intense rainfall event to determine the requirement of a culvert to reduce the risk of overflow.**	AWAR Road Culvert – Photo 55

Table 15-1: AWAR Road – Water Management Structures Summary

Station (distance from Friendship Centre)	Water Management Structure Description	Conditions, Observations, and Recommendations (at time of inspection)	Photo Page
KM 21.7	2 x 160 mm steel pipes, used as pipe sleeves	Vertically offset steel pipes, no flow. Water ponded upstream, erosion mark from higher water level evident in road embankment. Agnico Eagle personnel reported that the road was excavated in 2018 to allow the water to drain. Capacity of pipes may be inadequate, or pipes could have been frozen (blocked) causing water to backup. Outlets are blocked by the construction of the AWAR waterline, making the steel pipes no longer function. No reports of overflow in 2019 or 2020 or 2021. Pumping was required in 2021. High water marks in 2024/2025 does not indicate overflow occurred. **Recommendation: The drainage conditions at this location be monitored especially during the freshet period and after the heavy rainfall event. If the monitoring indicates that the placement of the culverts is required to improve the drainage, a mitigation plan should be developed and implemented.**	AWAR Road Culvert – Photos 56 and 57
KM 22.2	1 x 150 mm HDPE pipe, used as pipe sleeves	Pipe and embankments in good condition, erosion pits at outlet with no armouring present. High water marks on embankment show pipeline is functioning properly. No flow or ponding water is present. Pipe capacity may not be adequate under heavy precipitation events or an extreme freshet.	AWAR Road Culvert – Photos 58 and 59
KM 22.3	2 x 160 mm steel pipes, used as pipe sleeves	Ponded water observed in 2019, no armour around inlets. Inlets cannot be seen from the roadway and are covered by grass. The steel pipes are vertically offset with no flow. No indication of overflow. Agnico Eagle reported there was overflow at the location in 2020 and no overflow in 2021 and 2022 freshet. The outlet of the pipes was blocked and buried during the waterline project construction. **Recommendation: The drainage conditions at this location be monitored especially during the freshet period and after the heavy rainfall event. If the monitoring indicates that the placement of the culverts is required to improve the drainage, a mitigation plan should be developed and implemented.**	AWAR Road Culvert – Photos 60 and 61
KM 22.7 to 23.0	No culverts	Water ponded on the east side of the road. Distressed vegetation is an indication of some ponding. Required pumping to prevent the road from breaching in 2021. Observed high water level marks on upstream esker sand berm show no overflow. **Recommendation: The drainage conditions at this location be monitored especially during the freshet period and after the heavy rainfall event. If the monitoring indicates that the installation of the culverts is required to improve the drainage, a mitigation plan should be developed and implemented.**	AWAR Road Culvert – Photos 62 and 63

Table 15-1: AWAR Road – Water Management Structures Summary

Station (distance from Friendship Centre)	Water Management Structure Description	Conditions, Observations, and Recommendations (at time of inspection)	Photo Page
KM 26.1	1 x 600 mm HDPE corrugated culvert	No flow, gravel in base of culvert. Inlet is partially blocked by sandy soil from construction of the AWAR waterline and does not extend past the toe of the road. Sandy soil around, no armor. Minor erosion on slope of road. 2018 inspection reports noted ponding of water at or over the road in this area during the freshet. Deformation was observed inside the HDPE pipe, indicates that the HDPE culvert is not strong enough to carry the current traffic load. **Recommendation: The performance of the HDPE culvert at this location be monitored. If the monitoring indicates that the placement of the HDPE culvert is required, a mitigation plan should be developed and implemented.**	AWAR Road Culvert – Photos 64 to 65
KM 26.2	2 x 160 mm steel pipes, used as pipe sleeves	Vertically offset, lower pipe bent upward. The inlets are elevated close to the road surface. Some sediment deposition downstream is evident. Outlet of higher pipe is partially crushed. 2018 Agnico Eagle inspection reports noted ponding of water at or over the road in this area during the freshet. No reports of overflow in 2019. Ponding and overflow were reported during 2021/2022 freshet. No flow or ponding water.	AWAR Road Culvert – Photos 66 and 67
KM 26.6	3 x 700 mm CSP culverts	Equal elevation, minor sediment buildup, no flow, small dents, well armoured and covered with gravel. No signs of erosion. Outlet has exposed geotextile. Minor ponding water at inlet at time of the inspection.	AWAR Road Culvert – Photos 68 and 69
KM 26.8	2 x 160 mm steel pipes, used as pipe sleeves	Vertically offset, no flow. Evidence of ponding about 0.5 m below road crest. Agnico Eagle reports no overflow during 2021 freshet. The inlet of the steel pipes were buried during the waterline project construction and is blocked. High water marks and loss of fines were observed along the upstream embankment. **Recommendation: The drainage conditions at this location be monitored especially during the freshet period and after the heavy rainfall event. If the monitoring indicates that the placement of the culverts is required to improve the drainage, a mitigation plan should be developed and implemented.**	AWAR Road Culvert – Photos 70 and 71
KM 27.6	3 CSP culverts: 1 x 900 mm 1 x 700 mm 1 x 1,000 mm (southernmost)	Vertically offset, middle culvert (700 mm) elevated above adjacent culverts. Clear, minor flow in lowest culvert, some small dents in 900 mm and 1,000 mm culverts. All clear and in good condition.	AWAR Road Culvert – Photos 72 and 73

Table 15-1: AWAR Road – Water Management Structures Summary

Station (distance from Friendship Centre)	Water Management Structure Description	Conditions, Observations, and Recommendations (at time of inspection)	Photo Page
KM 28.7	No culverts	Ponded water on east side of road. 2018 water flowed over the road at this location during freshet. No reports of water flowing over the road in 2019 or 2020. Agnico Eagle reports pumping was required during 2021/2022. The construction of esker sand berm for the waterline project may reduce the risk of the overflow. The high-water mark on the berm indicates that about 1.0 m deep ponding water against the berm during the freshet season or extreme rainfall event. **Recommendation: The drainage conditions at this location be monitored especially during the freshet period and after the heavy rainfall event. If the monitoring indicates that the installation of the culverts is required to improve the drainage, a mitigation plan should be developed and implemented.**	AWAR Road Culvert – Photo 74
KM 29.6	Sump	Culvert Removed, water managed by pumping from a small sump. Water in the sump is pumped to CP5 for storage.	AWAR Road Culvert – Photo 75

15.2 AWAR Borrow Sources

Numerous borrow sources have been developed along the AWAR and have been used in the construction of the Meliadine Mine infrastructure, including the AWAR, and the AWAR waterline. The following borrow areas were inspected:

- KM 6.4 Borrow Source;
- KM 11.7 Borrow Source;
- KM 14.7 Borrow Source;
- KM 19.7 Borrow Source; and
- KM 24.7 Borrow Source.

Photographs of the borrow areas are presented in Photos 76 to 81 in Appendix T. The photo locations are presented in Figures 27, 29, 31, 32.

15.2.1 Visual Observations

In general, the borrow areas were in good condition, and no erosion or permafrost degradation was observed around them at the time of the inspection. The KM 6.4 Borrow Source is currently in use, with material being sourced and stockpiled.

15.2.2 Summary and Recommendations

Overall, the borrow sources appear to be performing well with no geotechnical concerns observed.

16.0 ITIVIA FUEL STORAGE SITE AND BYPASS ROAD

The Itivia bypass road is a 6.3 km gravel road that was constructed to divert traffic from the Itivia fuel storage and laydown area to the Meliadine Mine site around Rankin Inlet as shown in Appendix U. The Itivia fuel farm is used to store fuel for the Meliadine Mine. The photo locations are shown in Figures 20 to 24.

The road is designed to be 6.5 m wide for most of its length, with pull outs to allow two-way traffic. Two sections are designed to be 8 m wide to allow two-way traffic without pullouts. The road was constructed in 2017 and 2018. The eastern portion of the road was built with blast rock from the Itivia Quarry, while most of the road was built with esker materials.

The road and culvert locations were observed. The culvert locations are referenced from the southeast corner of the Itivia fuel storage facility. The observations are summarized in Table 16-1. The culvert names are referenced from the construction drawings and the 2018 inspection. Some of the culverts now have the names attached to the culverts, and do not correlate to the previous names as noted in Table 16-1.

In general, the road was in good condition. Minimal signs of cracking or settlement were noted. Some sections of the road were high enough to require safety berms, which were constructed using large boulders along the eastern section and with esker materials along the remainder of the road. Riprap was generally placed at the inlets and outlets of culverts, per the design. Table 16-1 summarizes the culvert inspections completed.

Based on discussions with Agnico Eagle personnel, it is understood that two areas had issues during the 2019 and 2020 freshets; the area northwest of Culvert C10 flooded, and the road at km 2 had significant flows in the upstream ditch running along the road, and across the road. The bypass road has not experienced any significant issues during the 2021 to 2025 freshets due to a combination of Agnico Eagle personnel performing snow removal and culvert steaming, and mild freshets.

Culvert C10 handles the flow of water from a small lake (Signet Lake) north of the road. In 2019, it appeared that most of the runoff ran along the road rather than flowing through the culverts. This is evidenced by the high-water mark on the road shoulder. The water ran to a low area of the road east of the culverts and then across the road. This may have been partially because of the icings around the culvert area in the spring. The road 200 m east of Culvert C10 was raised in 2019 to address this problem; however, the problem persisted in the spring of 2020. The Agnico Eagle Surface Water Superintendent reported in 2020 that the water partially originated from a discharge from Signet Lake and the southeast side. The problem could also have been partially caused by an ice/snow blockage in the C10 culverts. The culverts should always be cleared prior to each freshet event. The issue could be rectified by placing culverts in the low area of the road east of Culvert C10. It is understood that the culverts were steam cleaned in 2021 and 2022, and that the flow through the C10 culverts occurred as intended. Although the area functioned well with the mitigation activities applied prior to the freshet, culverts in the low area of C10 would reduce future problems with this area.

The road along km 2.2 has been constructed as a cross-slope fill. Water runs from the upgradient slope into a ditch upslope of the road. The ditch is relatively shallow (0.5 m). The water spills out of the ditch, runs across the road and down the road slope. It is recommended that the area be rectified to control the freshet water. This could be a combination of a culvert and improvements to the ditch. The solution must consider the steep upgradient slope, steep downstream erodible road fill, and shallow road fill at this location, making installation of a culvert difficult. The ditch should be cleared of snow and ice prior to each freshet event. This section of road did not experience any issues during the 2025 freshet according to Agnico Eagle personnel, but further development of the area should be done if future problems persist. No noticeable sign of permafrost degradation was observed along the road during the inspection.

During the 2025 annual geotechnical inspection, it was observed that the C11 culverts (3.1 km from the fuel tank farm) were upgraded. Details and conditions of the culverts are provided in Table 16-1.

Similar to the AWAR, it was noticed that some culverts do not have a sign for their easy identification and some culverts were mis-labelled. It is recommended that the signage for culverts be reviewed for accuracy and updated where required. Tetra Tech’s recommendations for the identification of culverts are listed in Table 16-1.

Table 16-1: Summary on Culverts on Itivia Bypass Road

Approximate Distance from SE Corner of Fuel Farm	Culvert Identification (Culvert Design Identification)	Water Management Structure Description	Observations	Photographs (Appendix U)
0.40 km	C01 (Design identification C01)	2 x 1,000 CSP culverts	Good condition, culverts vertically offset, no water flow. Inlet of lower culvert has been damaged, slightly reduced capacity. Road constructed out of blast rock. Large boulders placed on south crest of road as safety berm.	Itivia Bypass Road Culvert – Photos 1 to 3
0.65 km	C02 (Design identification C02)	2 x 700 mm CSP culverts	Good condition, culverts vertically offset, no water flow. Inlet of lower culvert has been damage, culvert capacity is unchanged, sandy soil used for culvert bedding material has entered the culvert where it is damaged. Additional erosion of the bedding material may occur. No water flowing through culverts. Road constructed out of blast rock. Large boulders placed on south crest of road as safety berm.	Itivia Bypass Road Culvert – Photos 4 to 6
0.85 km	C03 (Design identification C03)	2 x 1,000 mm 1 x 700 mm	Good condition, 700 mm culvert vertically offset. No water flowing through culverts. Minor erosion in tundra observed upstream of culverts. Road constructed out of blast rock. Large boulders placed on south crest of road as safety berm.	Itivia Bypass Road Culvert – Photos 7 to 9
1.0 km	C04 (Design identification C04)	2 x 1,000 mm	Good condition, eastern inlet damaged, inlet capacity is not affected. No flow in culverts. Minor amount of riprap upstream of culvert. Road constructed out of blast rock. Large boulders placed on south crest of road as safety berm.	Itivia Bypass Road Culvert – Photos 10 to 11
1.2 km	C05 (Design identification C05)	2 x 1,000 mm	Good condition, no water flowing through culverts. Road and safety berm on south crest of road constructed out of esker materials. Erosion booms are located at the inlet.	Itivia Bypass Road Culvert – Photos 12 and 14
1.5 km	C06 (Design identification C06)	2 x 800 mm	Good condition, culvert inlets installed above surrounding natural ground. A small berm has been constructed between the ponded water and the culvert inlet location. No water flowing through the culverts. Some rockfill in front of inlets could erode into the culverts. Road constructed out of esker materials. Erosion booms are located at the inlet.	Itivia Bypass Road Culvert – Photos 15 to 17
1.65 km	C07 (Design identification C06-1)	1 x 800 mm	Good condition, no water flow in culverts, road constructed out of esker materials, inlet and outlet covered with riprap. Erosion booms are located at the inlet.	Itivia Bypass Road Culvert – Photos 18 to 20
1.9 km	C08 (Design identification C07a)	2 x 800 mm	Good condition, ponded water observed at the inlets around the culverts and against the toe of the road embankment. No flow. Culvert inlets installed over rockfill base raised above surrounding natural ground. Road constructed out of esker materials. Erosion booms are located at the inlet.	Itivia Bypass Road Culvert – Photos 21 to 23
2.0 km	C09 (Design identification C07B)	2 x 1,000 mm	Good condition, no water flowing through the culverts, inlets and outlets are clear. Inlets are dented but appear to be functioning well. Road constructed out of esker materials. Safety berm constructed on south crest of road. Outlet discharges on coarse (cobbly) esker. Road crush has washed onto outlet culverts. Minor damage to inlet. Erosion booms are located at the inlet.	Itivia Bypass Road Culvert – Photos 24 to 26
2.6 km	C10 (Design identification C09)	2 x 1,000 mm	Good condition, no water flowing through culverts. Road constructed out of esker material. Small amount of erosion in armouring at inlet. Erosion booms are located at the inlet and geotextile was placed at the outlet to prevent erosion.	Itivia Bypass Road Culvert – Photos 27 to 29
3.4 km	C11 (Design identification C10)	2 x 2,000 mm	Some damage to culvert inlets and deformation under the road was observed. Some water flow observed flowing into lowest culvert. Ponded water observed along the toe of the road embankment to the north of the culvert inlets and at the culvert outlets. Culverts were steamed during freshet which prevented issues this year. Road constructed out of esker material. Road raised in fall 2019 north of culverts. During 2025, two 1,200 mm and one 1,000 mm diameter culvert were replaced with two 2,000 mm culverts lined with geotextile. During the replacement of the culverts the armour aprons and road embankments were reconstructed with proper armouring. Erosion booms are located at the inlet.	Itivia Bypass Road Culvert – Photos 30 to 32
4.25 km	C12 (Design identification C11a)	2 x 1,200 mm	Good condition, culverts are constructed over riprap and inverts are raised above surrounding tundra. No flow observed through both culverts. Shallow ponding of water over tundra upstream of the inlets and against the toe of the road embankment. Water mark visible along toe of road embankment, approximately 0.2 m above toe of road at culverts, higher along road to northeast. Road constructed out of esker material; minor erosion at toe of road.	Itivia Bypass Road Culvert – Photos 33 to 35
4.6 km	C13 (Design identification C11b)	2 x 1,000 mm	Good condition, culverts are constructed over riprap and inverts are raised above surrounding tundra. Shallow ponding water observed at inlets, minor water flow through the lowest culvert, appears to be performing well. Road constructed out of esker material, performing adequately, no signs of erosion.	Itivia Bypass Road Culvert – Photos 36 to 38
5.0 km	C14 (Design identification C11b-1)	1 x 1,000 mm	Good condition, invert of culvert is dented. Culvert inlet is damaged but is not expected to affect the culverts capacity. Culvert is constructed over riprap and invert is raised above surrounding tundra. Small natural drainage path observed upstream and downstream of culvert. No flow observed. Shallow ponding water at the culvert outlet. Road constructed out of esker material performing adequately.	Itivia Bypass Road Culvert – Photos 39 to 41
5.08 km	C15 (Design identification C11c)	2 x 1,200 mm	Good condition, culverts are constructed over riprap and inverts are raised above surrounding tundra. Minor flow going through the lowest culvert, ponding water observed at the inlet and outlet. Road constructed out of esker materials. Road fill performing adequately.	Itivia Bypass Road Culvert – Photos 42 and 43
5.2 km	C16 (Design identification C12a)	2 x 1,200 mm	Good condition, culverts are constructed over riprap and upstream inverts raised above surrounding tundra. No flow through the culvers, ponded water observed at the outlets. Small section of exposed geotextile was observed at the inlet of the northern culvert. Road constructed out of esker materials. Road slopes performing adequately.	Itivia Bypass Road Culvert – Photos 44 and 45
5.26 km	C17 (Design identification C12b)	2 x 1,000 mm	Good condition, culverts are constructed over riprap and inverts are raised above surrounding tundra. No ponded water or flow observed. Road constructed out of esker materials. Riprap placed in local area of culvert. No signs of erosion on roadside slopes indicating previous higher water levels.	Itivia Bypass Road Culvert – Photos 46

Table 16-1: Summary on Culverts on Itivia Bypass Road

Approximate Distance from SE Corner of Fuel Farm	Culvert Identification (Culvert Design Identification)	Water Management Structure Description	Observations	Photographs (Appendix U)
6.3 km	C18 (Design identification C13)	2 x 800 mm	Good condition, culverts are constructed over riprap and inverts are raised above surrounding tundra. Relatively large pond of water upstream and downstream of the culverts; road constructed through natural pond. Culvert inlets and outlets are dented, no flow observed. The west inlet is damage with the culvert being bent with ¼ of the area. Road constructed out of esker materials.	Itivia Bypass Road Culvert – Photos 47 to 49
6.5 km	C19 (Design identification C14)	3 x 800 mm	Good condition, culverts are constructed over riprap and inverts are raised above surrounding tundra. Water ponded upstream and downstream of the culverts. No signs of subsidence due to ponded water. No water flow. Road constructed out of esker materials. Left inlet culvert has a dent in the middle, underneath the road fill.	Itivia Bypass Road Culvert – Photos 50 to 52

The Itivia fuel farm consists of four fuel storage tanks, with two constructed in 2017 and two in 2024. (Photos 1 through 7, Appendix U). The tanks have storage capacity of 4.5 ML, 9 ML, 13.5 ML, and 20 ML, for a total storage capacity of 47 ML. The fuel is hauled to the Meliadine Mine on an as needed basis. The tanks are contained within a geomembrane-lined containment facility. The geomembrane liner is covered with a layer of geotextile and 20 mm crushed rock. The following observations were made during the inspection.

- Liner and berms were raised to accommodate construction of additional fuel tanks.
- Ponding water was observed at the southeast corner of the tank farm and in small localized areas (Photos 1 and 3).
- Localized depressions were observed within the tank farm floor (Photo 7).
- The edge of one tank pedestal has minor surface erosion of the granular crush.
- Minor cracking was observed along the northeast perimeter berm (Photo 6).

In general, the facility appears to be in good condition from a geotechnical perspective. Minor erosion of the granular fill pedestals should be monitored and addressed by building up the pedestals to prevent further development of erosion channels. Water in the facility should be emptied as soon as practical to reduce the risk of erosion. Coarser rockfill could be placed adjacent to the narrow points of the pedestals to reduce the risk of erosion. The minor cracking along the northeast perimeter berm should be monitored for future cracking and settlement.

17.0 SUMMARY OF RECOMMENDATIONS

A total of 15 recommendations were made during the 2024 annual geotechnical inspection, including eight from the 2024 annual geotechnical inspection and seven carried over from the 2023 annual geotechnical inspection. Of the 15 recommendations made during the 2024 annual geotechnical inspection, six recommendations were executed in 2025 and nine noted as in progress and carried over to 2025. Tetra Tech understands that Agnico Eagle established a systematic monitoring procedure and operational management plans to monitor the performance of each infrastructure at the Meliadine Mine. The generic recommendations, for example, “continuing to monitor the performance of the structure to determine the requirement of mitigation”, were excluded. Only the recommendations that are beyond the daily operations/maintenance/surveillance scope and require additional actions are included in this section.

A total of 16 recommendations are made based on the 2025 annual geotechnical inspection, which include seven new recommendations and nine carry-overs from 2024. A priority level has been assigned to each recommendation related to critical infrastructure, based on the priority scale system as described in Section 2.

Table 17-1 summarizes recommendations based on observations from the 2025 annual geotechnical inspection.

Table 17-1: Summary of Recommendations

Section	Structure/Facility	Priority	Recommendations	Status
5.2.5	Dike D-CP1	P2	It is recommended that snow removal and snow fencing be continued while investigating other mitigation measures. A thermal performance review, thermal model calibration, and the development of a mitigation plan should be conducted.	Carried over from the 2024 inspection.
5.8.4	Berm4	P3	Depression area between WRSF6 and Berm4, where ponding water was observed, should be monitored to determine the requirement of mitigation.	New from the 2025 inspection.
7.3	Channel1	P3	Complete the repair and maintenance work at the lower reach of Channel1.	Partially completed. Carried over from the 2024 inspection.
7.3	Channel5	P3	It is recommended that the eastern half of Channel5 be repaired to the design grade.	Carried over from the 2024 inspection.
7.3	Berm3	P2	Repairs and maintenance should be conducted along the crest of Berm3 to remediate cracking.	New from the 2025 inspection.
8.5	TSF	P3	It is recommended that regrading of the rockfill slope be completed at the southwest corner of the TSF.	New from the 2025 inspection.
8.5	TSF	N/A	It is recommended that a 4 Horizontal to 1 Vertical slope be maintained along the north slope of Cell 2.	New from the 2025 inspection.
8.5	TSF	P4	Consideration should be given to backfilling the ponded water area along the east toe of the TSF.	New from the 2025 inspection.
9.2	WRSF3	P3	Regular pumping during the operations should continue until other mitigation measures are implemented.	Carried over from the 2024 inspection. (ongoing recommendation)
13.7	Portal No. 2	N/A	It is recommended that the voids underneath the footing foundations that support the corrugated steel entry of Portal No. 2 are backfilled, and erosion protection measures are put in place to prevent additional erosion along the base of the footing.	Carried over from the 2024 inspection.
13.8	Mine Site Fuel Tank Farm	N/A	It is recommended that additional liner cover material be placed over the exposed liner area on the downstream side of the perimeter berm to prevent liner cover material further sliding, and the downstream slope be regarded to the design grade.	New from the 2025 inspection.
13.9	Berm DP3-A	N/A	It is recommended that the cracking observed on the top of the berm be monitored and assessed, and that a mitigation plan be developed, if required.	New from the 2025 inspection.
15.1	AWAR	N/A	Monitor the drainage condition where the steel pipes (serves as pipe sleeves) are blocked due to the new waterline, if there is ponding water, risk of overflow, or washouts then new culverts should be installed.	Carried over from the 2024 inspection. (ongoing recommendation)
16.0	Itivia Bypass Road	N/A	Several low-lying areas should be monitored for potential overflow; additional culverts be installed where required.	Carried over from the 2024 inspection. (ongoing recommendation)
		N/A	Water in the Itivia tankfarm facility should be emptied as soon as practical to reduce the risk of erosion. Consideration should be given to fill and regrade the areas with depressions.	Carried over from the 2024 inspection. (ongoing recommendation)
Multiple	Site Culverts	N/A	It is recommended that the signage for culverts and KM stations be reviewed for accuracy and updated where required.	Partially completed. Carried over from the 2024 inspection.

N/A: Not applicable to the the priority scale system related to the critical infrastructure.

18.0 CLOSURE

We trust this report meets your present requirements. If you have any questions or comments, please contact the undersigned.

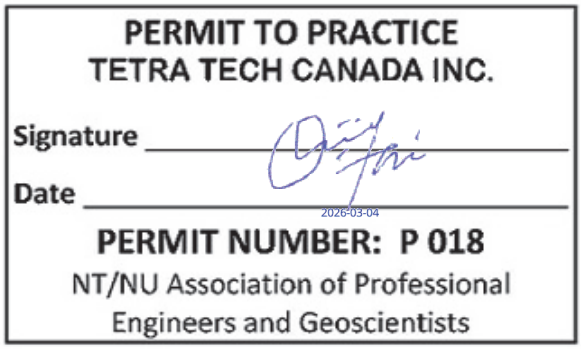
Respectfully Submitted,
Tetra Tech Canada Inc.

2026-03-04

Prepared by:
Devon Sosniuk, B.Sc., E.I.T.
Geotechnical Engineer-in-Training, Arctic Group
Direct Line: 587.460.3664
Devon.Sosniuk@tetratech.com

Reviewed by:
Hongwei Xia, Ph.D, P.Eng.
Principal Specialist /Senior Geotechnical Engineer, Arctic Region
Direct Line: 587.460.3646
Hongwei.Xia@tetratech.com

2026-03-04



Reviewed by:
Fai Ndofor, M.Sc., P.Geo.
Principal Consultant, Arctic Region
Direct Line: 587.460.3486
Fai.Ndofor@tetratech.com

/jf

REFERENCES

Agnico Eagle Mines Limited (Agnico Eagle). 2014a. Meliadine Gold Project, Nunavut. Final Environmental Impact Statement. Submitted to the Nunavut Impact Review Board. April 2014.

Agnico Eagle. 2014b. Volume 6, Supporting Document 6-1 – Permafrost Thermal Regime Baseline Studies. Final Environmental Impact Statement (FEIS) – Meliadine Gold Project, Nunavut. Submitted to Nunavut Impact Review Board. April 2014.

Agnico Eagle. 2015a. Risk Management and Emergency Response Plan. Version 4. April 2015. Document No. 6513-RMMM-01.

Agnico Eagle. 2015b. Roads Management Plan. Version 4. April 2015. Document No. 6513-MPS-03.

Agnico Eagle. 2016a. Letter regarding: Meliadine - 2BB-MEL1424, Meliadine Portal Area – Water Quality Monitoring in Lake A54, Adverse Trends Noted – Mitigation Measures being implemented. Submitted to Nunavut Water Board and Kivalliq Inuit Association. March 17, 2016.

Agnico Eagle. 2019. Construction Summary Report (As-built) Report for Berm2, Channel 3, Pond CP3, and Berm CP-3. October 2019. Document Number 6515-695-230-REP-001.

Agnico Eagle. 2019. Construction Summary Report (As-built) Report for Channel 4, Pond CP4, and Berm CP-4. December 2019. Document Number 6515-695-230-REP-002.

Agnico Eagle. 2021. Water Management Plan. Version 2. March 2018. Document No. 6513-MPS-11.

Agnico Eagle. 2024. Updated Operation, Maintenance, and Surveillance (OMS) Manual. December 2024.

Agnico Eagle. 2025. Operation, Maintenance, and Surveillance (OMS) Manual for the Tailings Storage Facility (TSF). April 2025.

CDA (Canadian Dam Association). 2013. Dam safety guidelines. Original 2007, revised 2013.

Golder, 2012a. SD 6-1 Permafrost Thermal Regime Baseline Studies – Meliadine Gold Project, Nunavut, Canada. A Technical Report Submitted to Agnico Eagle Mines Ltd. by Golder Associates, September 25, 2012.

Golder. 2014. SD 6-1 Permafrost Thermal Regime Baseline Studies – Meliadine Gold Project, Nunavut. Report submitted to Agnico Eagle Mines Limited. Golder Doc. No. 255-1314280007 Ver. 0. April 2014.

Golder. 2017. Meliadine Gold Project, Rankin Inlet, Nunavut. 2016 Annual Geotechnical Inspection. Report submitted to Agnico Eagle Mines Limited. Golder Doc. No. 1660296-006-R-Rev0-2000. February 2017.

Golder. 2018. 2019 Annual Geotechnical Inspection, Meliadine Gold Project, Rankin Inlet, Nunavut. Submitted to: Agnico Eagle Mines Limited. November 2018.

NRC (National Research Council Canada). 1993. The Atlas of Canada Permafrost. http://atlas.nrcan.gc.ca/auth/english/maps/environment/land/permafrost.

Nunavut Water Board. 2020. Water Licence No. 2AM-MEL1631. Issued 1 April 2020.

Tetra Tech EBA Inc. (Tetra Tech EBA). 2016a. Design Report for D-CP1 Meliadine Gold Project, NU. Prepared for Agnico Eagle Mines Limited. August 15, 2016. Document No. 6515-E-132-007-132-REP-003.

Tetra Tech EBA. 2016b. Design Report for D-CP5 Meliadine Gold Project, NU. Prepared for Agnico Eagle Mines Limited. August 15, 2016. Document No. 6515-E-132-007-132-REP-004.

Tetra Tech EBA. 2016c. Meliadine – P-Area Containment Construction Record Report. Prepared for Agnico Eagle Mines Limited. September 2016. Document No. 6515-C-230-002.

Tetra Tech EBA. 2016d. Design Report for Berm 3 and Channel 5, Meliadine Gold Project, NU. Prepared for Agnico Eagle Mines Limited. September 2016. Document No. 6515-E-132-007-132-REP-005.

Tetra Tech EBA. 2016e. Design Report for Wesmeg Road Culverts Meliadine Project, Nunavut. Prepared for Agnico Eagle Mines Limited. October 2016. Document No. 6515-E-132-005-132-REP-003.

Tetra Tech. 2016f. Construction Summary Report for Culvert 3 and Culvert 4, Meliadine Project, Nunavut. Prepared for Agnico Eagle Mines Ltd. December 9, 2016. Document No. 6515-E-132-005-132-REP-005.

Tetra Tech. 2017a. Construction Summary (As-Built) Report for Saline Water Storage/Transfer Pond and Berm, Meliadine Gold Project. Prepared for Agnico Eagle Mines Limited. February 3, 2017. Document No. 6515-E-132-007-132-REP-006.

Tetra Tech. 2017b. Design Report and Drawings for Rankin Inlet Bypass Road, Culverts and Bridge, Meliadine Gold Project. Prepared for Agnico Eagle Mines Limited. March 2017. Document No. 6515-E-132-005-132-REP-006.

Tetra Tech. 2017c. Design Report for Operation Landfill (Stage 1), Meliadine Gold Project. Prepared for Agnico Eagle Mines Limited. July 13, 2017. Document No. 6515-E-132-007-132-REP-010.

Tetra Tech. 2017d. Amendment#1 To 6515-E-132-005-132-REP-006: Design report for Crossing C10 on Rankin Inlet Bypass Road Meliadine Project, Nunavut. Prepared for Agnico Eagle Mines Limited. July 28, 2017. Document No. Amendment#1 To 6515-E-132-005-132-REP-006.

Tetra Tech. 2017e. Construction Summary (As-built) Report for Channel 2, Meliadine Project, Nunavut. Prepared for Agnico Eagle Mines Ltd. July 28, 2017. Document No. 6515-C-230-005-230-REP-003.

Tetra Tech. 2017f. Amendment#2 To 6515-E-132-005-132-REP-006: Design Report for Rankin Inlet Bypass Road Culverts C13 and C14 Meliadine Project, Nunavut. Prepared for Agnico Eagle Mines Limited. September 27, 2017. Document No. Amendment#2 To 6515-E-132-005-132-REP-006.

Tetra Tech. 2017g. Construction Summary (As-Built) Report for Dike D-CP5, Meliadine Gold Project Nunavut. Prepared for Agnico Eagle Mines Limited. October 6, 2017. Document No. 6515-E-132-007-132-REP-013.

Tetra Tech. 2017h. Construction Summary (As-Built) Report for Dike D-CP1, Meliadine Gold Project Nunavut. Prepared for Agnico Eagle Mines Limited. October 19, 2017. Document No. 6515-E-132-007-132-REP-012.

Tetra Tech. 2017i. Construction Summary (As-Built) Report for Jetty-CP1 and Jetty-CP5, Meliadine Project, Nunavut. Prepared for Agnico Eagle Mines Ltd. November 13, 2017. Document No. 6515-E-132-005-132-REP-012.

Tetra Tech. 2018a. Construction Summary (As-built) Report for Landfarm, Meliadine Project, Nunavut. Prepared for Agnico Eagle Mines Ltd. February 8, 2018. Document No. 6515-E-132-007-132-REP-016.

Tetra Tech. 2018b. Construction Summary (As-built) Report for Mine Site Fuel Storage and Containment Facilities, Meliadine Project, Nunavut. Prepared for Agnico Eagle Mines Ltd. April 17, 2018. Document No. 6515-E-132-005-132-REP-016.

Tetra Tech. 2019. Email from J. Alaire (Tetra Tech) to M. Long (Agnico). August 29, 2019.

Tetra Tech and WSP Canada Inc. 2017. Meliadine Project Design Report and Drawings for the Fuel Storage and Containment Facilities. Prepared for Agnico Eagle Mines Limited. April 2017. Document No. 6515-E-132-004-132-REP-006.

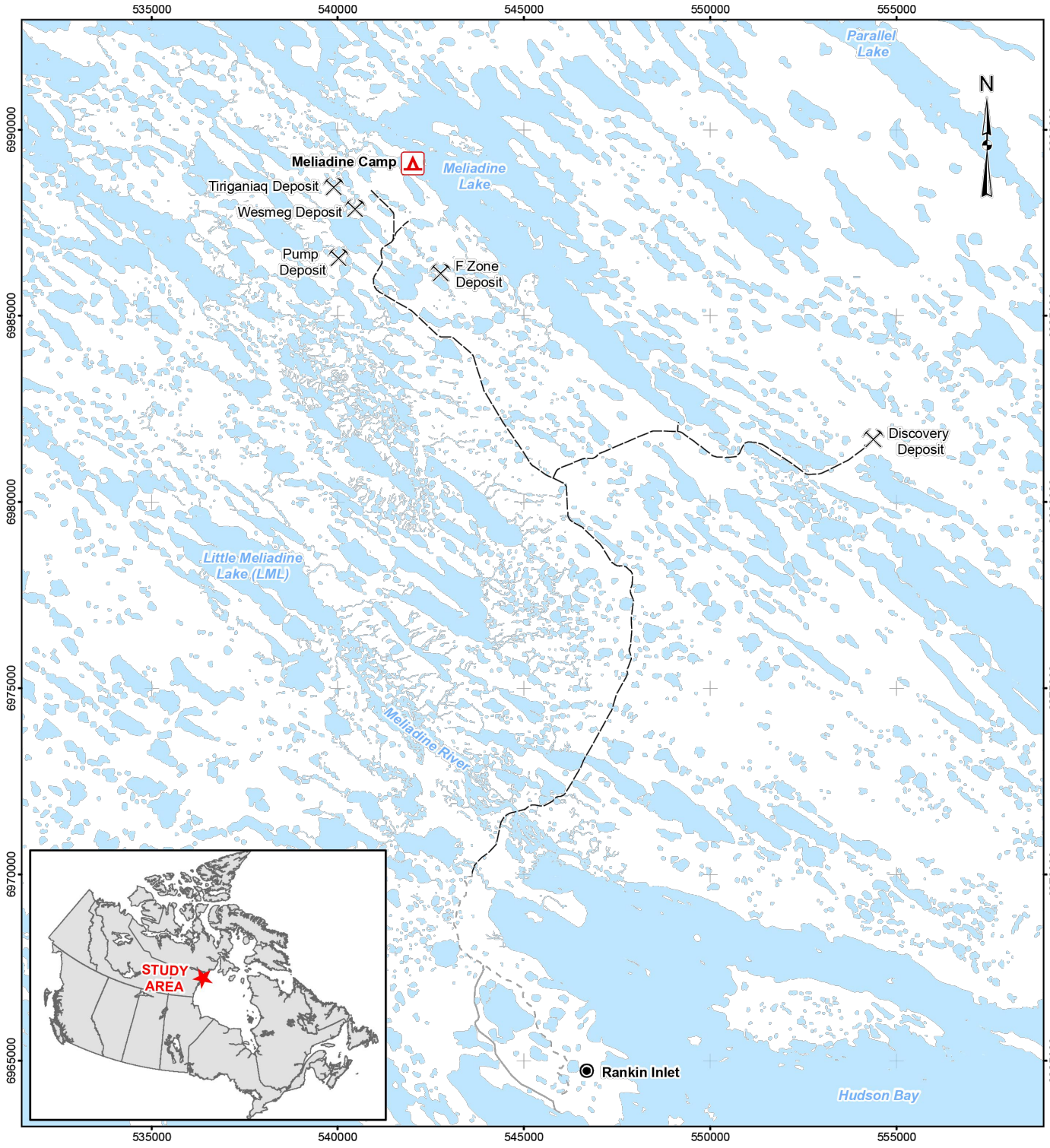
Tetra Tech. 2022. Construction Summary (As-built) Report for CP2 Pond, CP2 Berm, Channel9, and Channel10, Meliadine Project, Nunavut. Prepared for Agnico Eagle Mines Ltd. October 14, 2022. Document No. 6537-695-230-REP-002.

Tetra Tech. 2025. Construction Summary (As-Built) Report for Channel11, CP9 Thermal Berm, and Berm4, Meliadine Gold Mine, NU. Prepared for Agnico Eagle Mines Limited. August 5, 2025. Document No. 65-695-230-TCR-001.

WSP Canada Inc. (WSP), 2024a. Meliadine Mine – 2022 Thermal Assessment. A technical report submitted by WSP Canada Inc. to Agnico Eagle Mines Ltd. on January 12, 2024. WSP file: 22513890-938-R-REV1.

WSP, 2024b. Meliadine Mine – Updated Hydrogeology Modelling. A technical report submitted by WSP Canada Inc. to Agnico Eagle Mines Ltd. on January 24, 2024. WSP file: CA0020476.6818-MEL2024_004-R-REV0.

FIGURES



LEGEND

- Camp
- Proposed Mine Site
- All-weather Access Road (AWAR)
- Road - New
- Road - Existing
- Watercourse
- Waterbody

AGNICO EAGLE – MELIADINE DIVISION

AGNICO EAGLE

TETRA TECH

AGNICO EAGLE – MELIADINE GOLD MINE

FIGURE 1 GENERAL PROJECT LOCATION PLAN

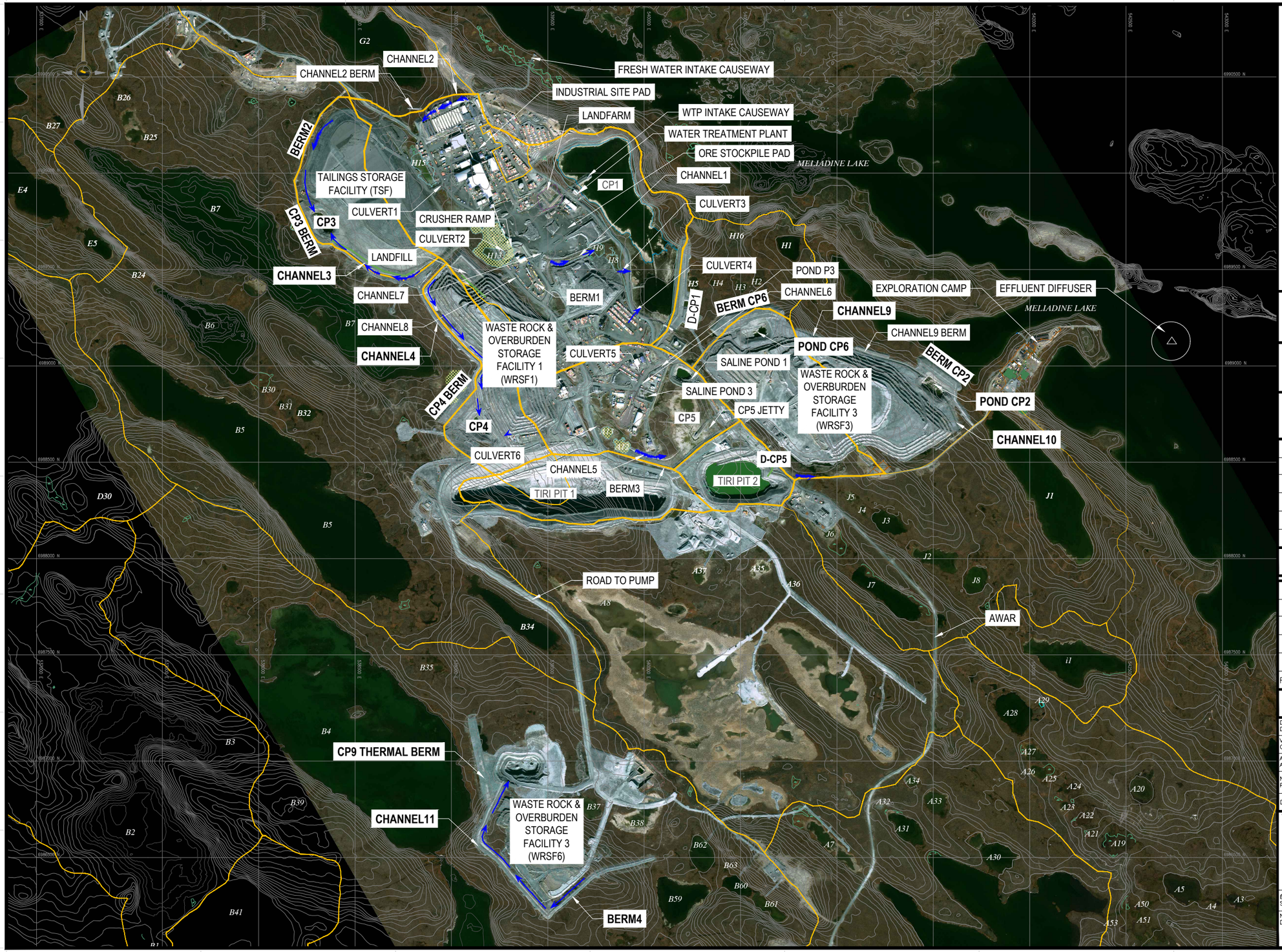
No. PROJET PROJECT No.	EARC03140-46	DATE	10-02-2025
DESSINÉ PAR DRAWN BY	DS		FEUILLE/SHT 1 / 1
APPROUVÉ PAR APPROVED BY	HX		
NO. DESSIN DRAWING NO.			REVISION 0

Q:\Edmonton\Engineering\E141\Projects_MELIADINE\ENG.EARC03140-46 – 2025 Annual Inspection\Figures\Figure 1.dwg

FILE NO. .DWG

NO. DESSIN
DRAWING NO.

FORMAT ANSI B–11x17



NOTE

1. PHOTO FROM SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE
2. CONTOURS PRIOR TO SITE DEVELOPMENT

LEGEND

- CATCHMENT BOUNDARY
- WATER FLOW DIRECTION
- DRAINED LAKE AREA
- WATER COLLECTION POND

0 1 000 m

Scale: 1: 20 000

AGNICO EAGLE

TETRA TECH

–	–
–	–
–	–
–	–
TITRE / TITLE	# DWG

DESSINS EN RÉFÉRENCE/REFERENCE DRAWINGS

REV	DESCRIPTION	DATE	PAR BY
A	ISSUED FOR REVIEW	2025–12–17	HX

REVISIONS

DESSINÉ PAR DRAWN BY	DS	DATE	2025–12–17
VÉRIFIÉ PAR CHECKED BY	HX		2025–12–17
APPROUVÉ PAR APPROVED BY			
No. PROJET PROJECT NO.	ENG.EARC03140–46		
DATE			

TITRE / TITLE

AGNICO EAGLE – MELIADINE GOLD MINE

FIGURE 2 – MELIADINE MINE 2025 GENERAL SITE LAYOUT

ÉCHELLE/ SCALE	1:20000	FICHIER FILE	General Site Layout Plan .DWG
No. DESSIN/ DRAWING NO.		REVISION: A	FEUILLE/SHT: 1 / 1

AE–CART–ANSI B

Q:\Edmonton\Engineering\E141\Projects_MELIADINE\ENG.EARC03140–46 – 2025 Annual Inspection\Figures\Figure 2 – General Site Layout.dwg

LEGEND:

1 - PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

D - CP1 PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	December, 2025

FIGURE 3

LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

CP2, CHANNELS 9 AND 10 PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	December, 2025

FIGURE 4

LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT
AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

CP3 AND CHANNEL 3 PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	December, 2025

FIGURE 5

LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT
AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

CP4 AND CHANNEL 4 PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0
OFFICE: EDM	DATE: December, 2025		

FIGURE 6

LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

D - CP5 AND CP5 PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0
OFFICE: EDM	DATE: December, 2025		

FIGURE 7

Q:\Edmonton\Engineering\E141\Projects_MELIADINE\ENG.EARC03140-46 - 2025 Annual Inspection\Figures\Photo Locations 1_IFR.dwg [FIGURE 7] January 23, 2026 - 1:02:12 pm (BY: SOSNILIK, DEV,ON)

LEGEND:

1 - PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

CP6 AND THERMAL BERM PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	December, 2025

FIGURE 8

Q:\Edmonton\Engineering\E141\Projects_MELIADINE\ENG.EARC03140-46 - 2025 Annual Inspection\Figures\Photo Locations 1_IFR.dwg, [FIGURE 8], January 23, 2026 - 1:02:19 pm (BY: SOSNIUK, DEVON)

LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT
AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

CP9 THERMAL BERM, CHANNEL 11, AND BERM 4 PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0
OFFICE: EDM	DATE: December, 2025		

FIGURE 9

Q:\Edmonton\Engineering\E141\Projects_MELIADINE\ENG.EARC03140-46 - 2025 Annual Inspection\Figures\Photo Locations 1_IFR.dwg [FIGURE 9], January 23, 2026 - 1:02:22 pm (BY: SOSNILIK, DEVON)

LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

SALINE PONDS PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	December, 2025

FIGURE 10

CHANNEL2

CHANNEL2 BERM

Channel2 - Photo 8

Channel2 - Photo 7

Channel2 Berm - Photo 20

Channel2 - Photo 6

CAMP AREA

Channel1 - Photo 2

Channel1 - Photo 1

Channel1 - Photo 3

Channel1 - Photo 4

Channel1 - Photo 5

PORTAL 2

Channel7 - Photo 14

LANDFILL

ORE PILES

Channel7 - Photo 13

LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

CHANNELS AND BERMS PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0
OFFICE: EDM	DATE: December, 2025		

FIGURE 11

Q:\Edmonton\Engineering\E141\Projects_MELIADINE\ENG.EARC03140-46 - 2025 Annual Inspection\Figures\Photo Locations 1_IFR.dwg [FIGURE 11], January 23, 2026 - 10:36 pm (BY: SOSNIUK, DEVON)

LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

TSF PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE: EDM

DATE: December, 2025

FIGURE 12

Q:\Edmonton\Engineering\E1411\Projects_MELIADINE\ENG.EARC03140-46 - 2025 Annual Inspection\Figures\Photo Locations 1_IFR.dwg [FIGURE 12] January 23, 2026 - 1:02:42 pm (BY: SOSNIUK, DEVON)

LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

WRSF1 PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	December, 2025

FIGURE 13

LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

WRSF3 PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	December, 2025

FIGURE 14

LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT
AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

WRSF6 PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	December, 2025

FIGURE 15

LEGEND:

1 - PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

SITE ROAD PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	December, 2025

FIGURE 16

Q:\Edmonton\Engineering\E141\Projects_MELIADINE\ENG.EARC03140-46 - 2025 Annual Inspection\Figures\Photo Locations 1_IFR.dwg [FIGURE 16] January 23, 2026 - 1:03:04 pm (BY: SOSNIUK, DEVON)

MELIADINE
NORTH ESKER

Meliadine North Esker - Photo 3

Meliadine Esker - Photo 2

Meliadine Esker - Photo 1

MELIADINE ESKER

TSF (CELL2)

CP1

TSF (CELL1)

Ore Stockpile - Photo 2

ORE STOCKPILES

Ore Stockpile - Photo 1

LAKE B7

WRSF1

N

LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

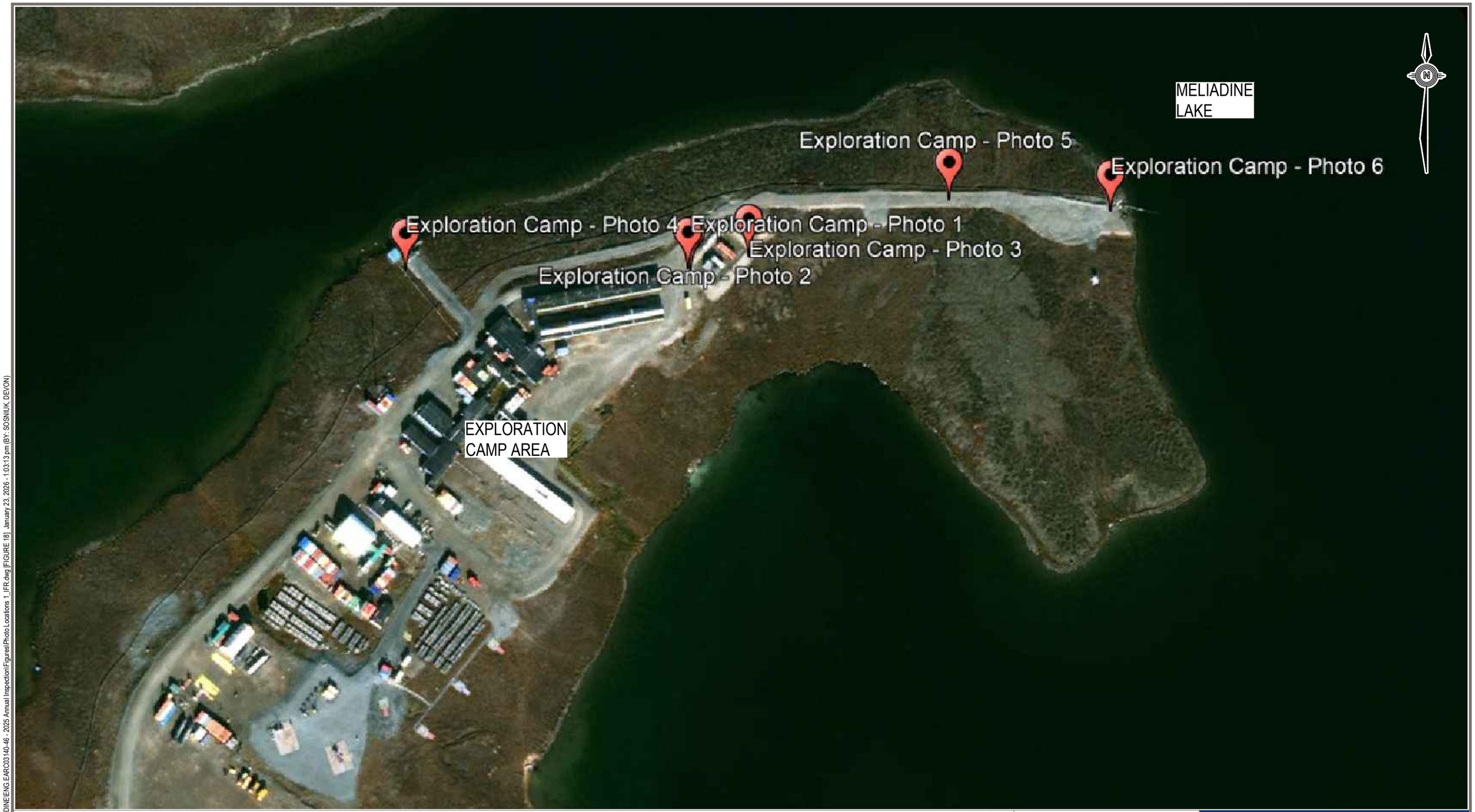
BORROW SOURCES AND ORE PILES PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	December, 2025

FIGURE 17

Q:\Edmonton\Engineering\E1411\Projects_MELIADINE\ENG.EARC03140-46 - 2025 Annual Inspection\Figures\Photo Locations 1_IFR.dwg [FIGURE 17], January 23, 2026 - 1:03:07 pm (BY: SOSNIUK, DEVON)



LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

EXPLORATION CAMP PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	December, 2025

FIGURE 18

LEGEND:

- **PHOTO LOCATION**

NOTE:
SATELLITE IMAGE TAKEN ON SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

OTHER FACILITIES PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0
OFFICE: EDM	DATE: December, 2025		

FIGURE 19a

C:\Users\DEVON.SOSNIUK\Documents\2025 MEL Annual Inspection Figures\Photo Locations 1-IFR.dwg [FIGURE 19A] March 04, 2026 - 10:03:17 am (BY: SOSNIUK, DEVON)

LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT
AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

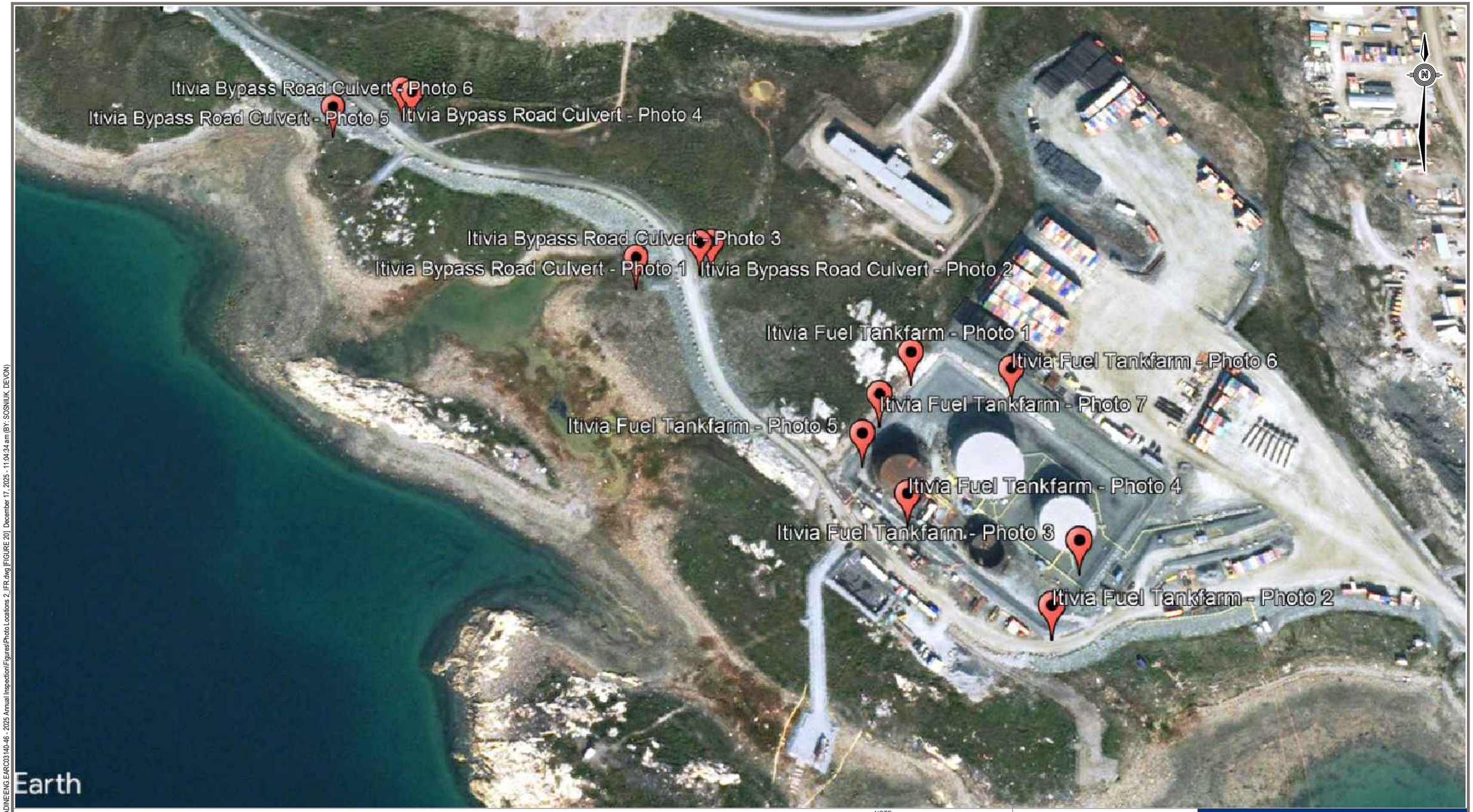
OTHER FACILITIES PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	March, 2026

FIGURE 19b

C:\Users\DEVON.SOSNIUK\Documents\2025 MEL Annual Inspection Figures\Photo Locations 1-IFR.dwg [FIGURE 19B] March 04, 2026 - 10:03:48 am (BY: SOSNIUK, DEVON)



LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON AUGUST 10, 2024, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

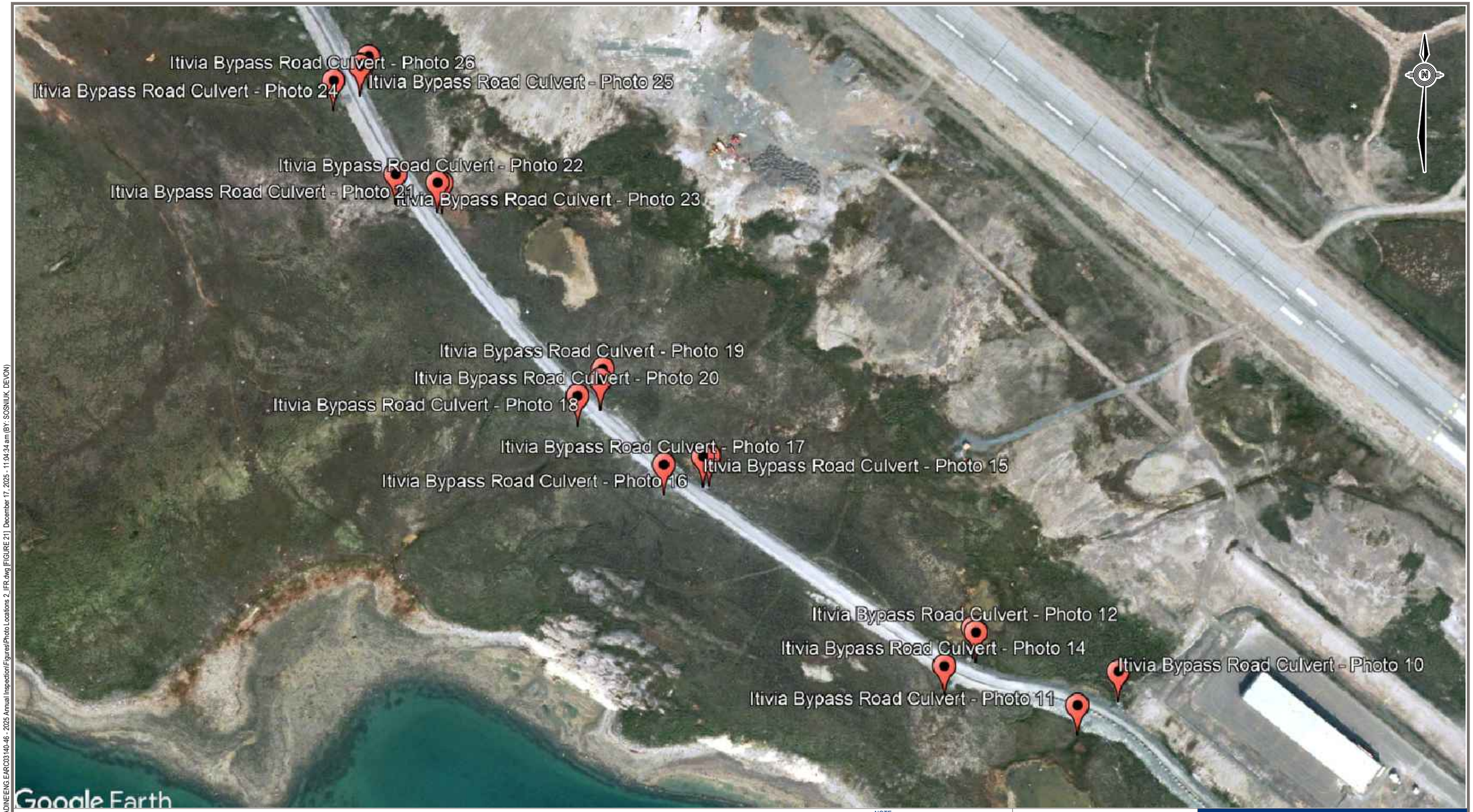
TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

ITIVIA BYPASS ROAD AND CULVERT PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0
OFFICE: EDM	DATE: December, 2025		

FIGURE 20



LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON AUGUST 10, 2024, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

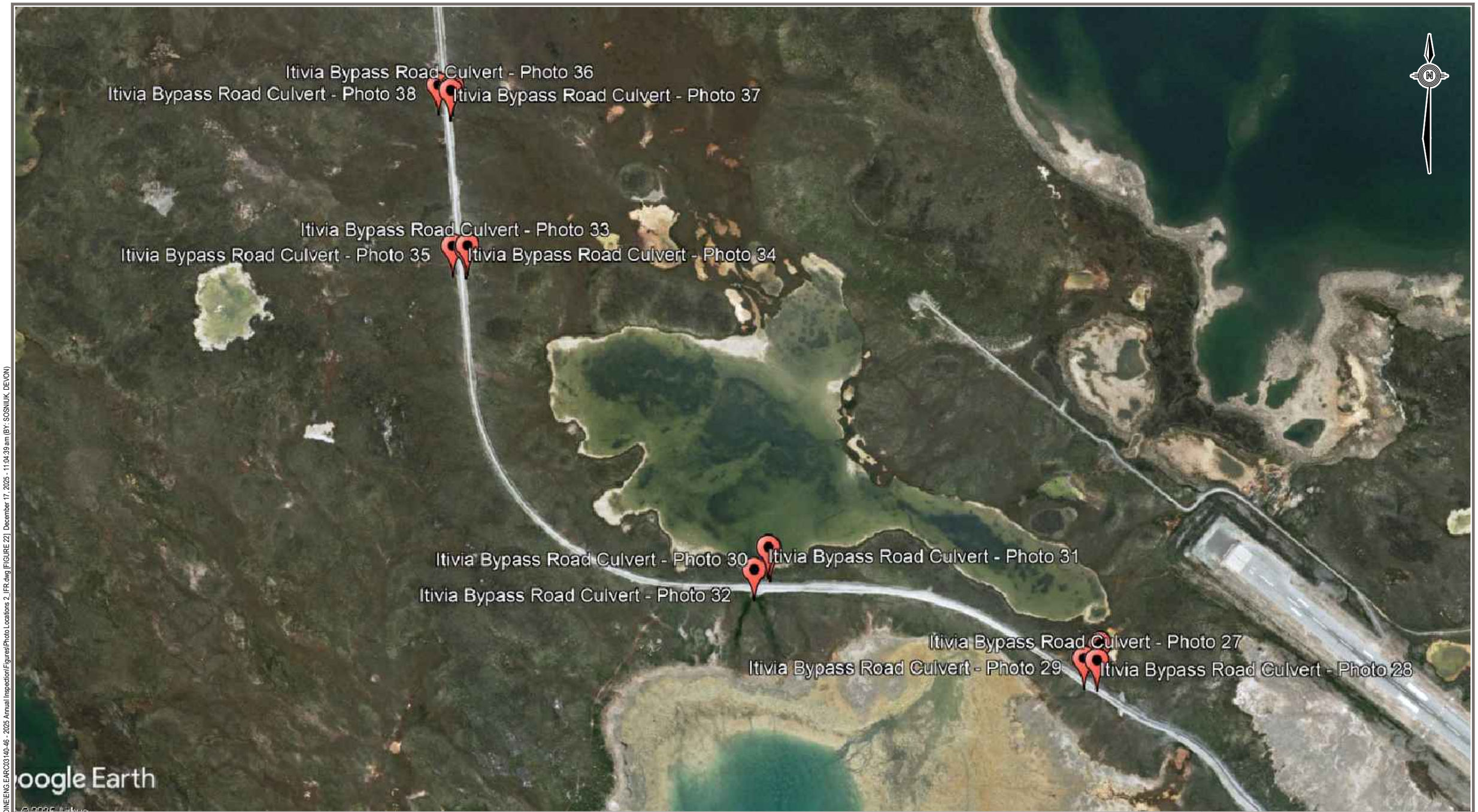
MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

ITIVIA BYPASS ROAD AND CULVERT PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	December, 2025

FIGURE 21



LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON AUGUST 10, 2024, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

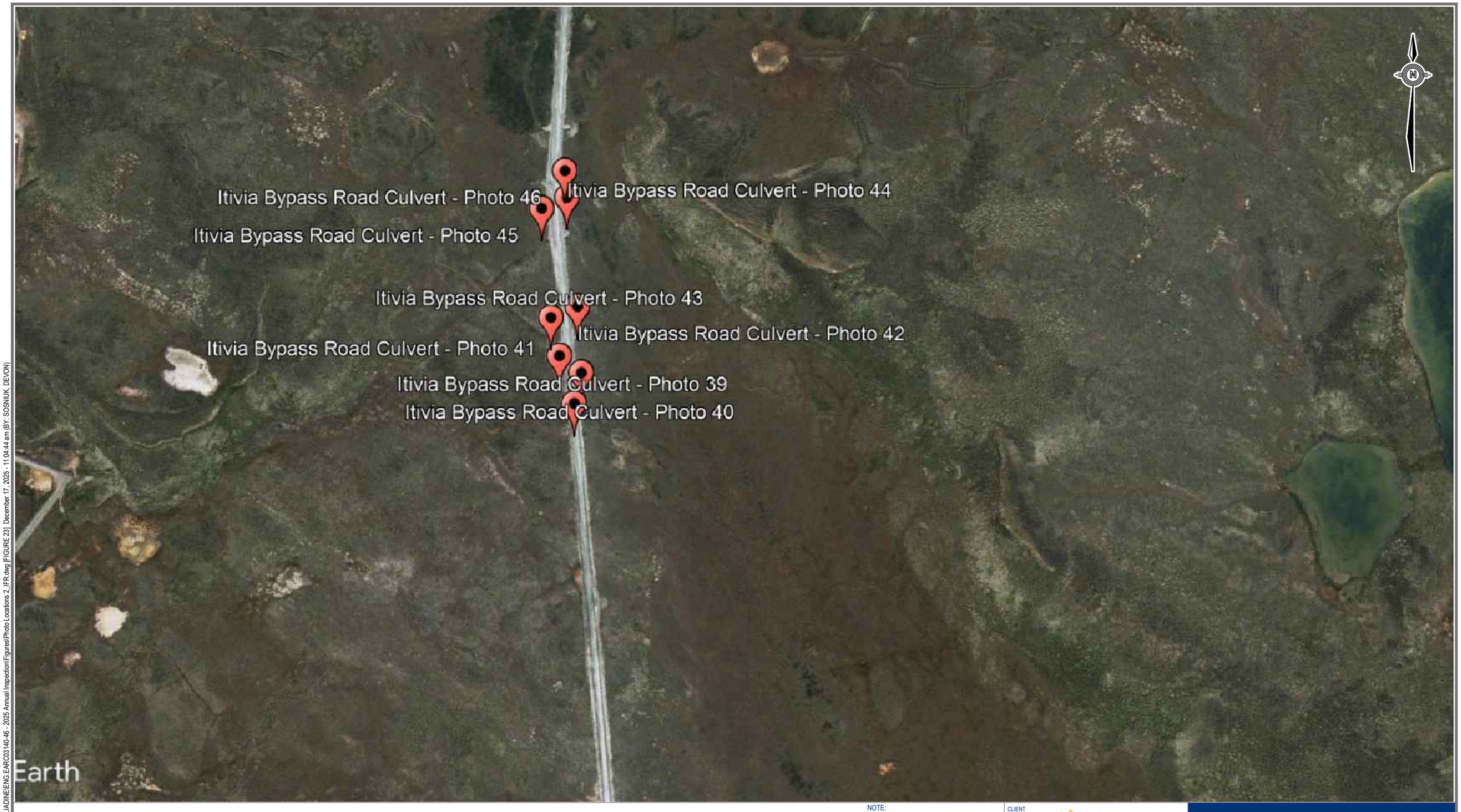
ITIVIA BYPASS ROAD AND CULVERT PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	December, 2025

FIGURE 22

Q:\Edmonton\Engineering\E141\Projects_MELIADINE\ENG.EARC03140-46 - 2025 Annual Inspection\Figures\Photo Locations 2_IFR.dwg [FIGURE 22] December 17, 2025 - 11:04:39 am (BY: SOSNIUK, DEVON)



LEGEND:

1 - PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON AUGUST 10, 2024, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

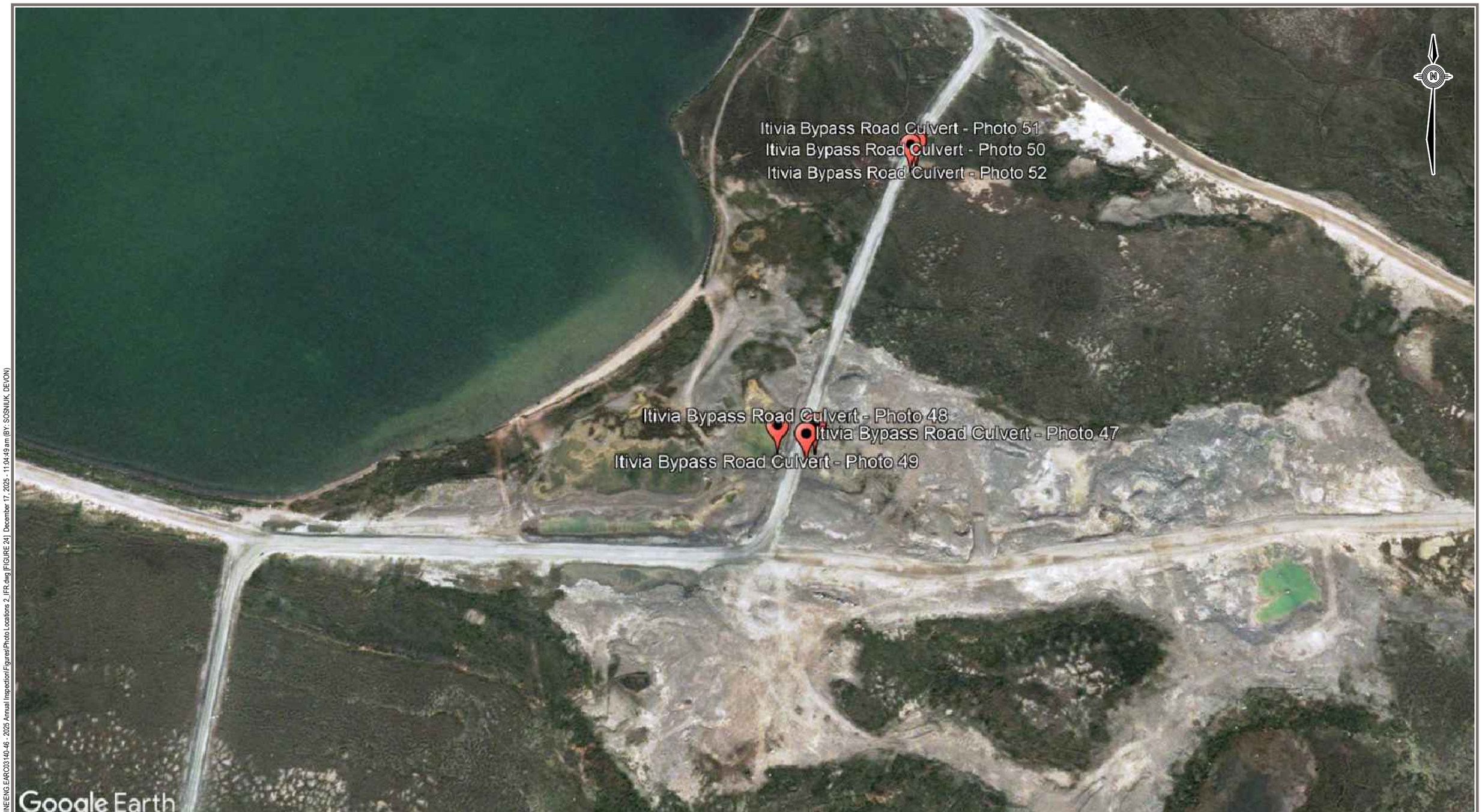
MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

ITIVIA BYPASS ROAD AND CULVERT PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-39	DS	HX	0

OFFICE	DATE
EDM	December, 2024

FIGURE 23



LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON AUGUST 10, 2024, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT
AGNICO EAGLE

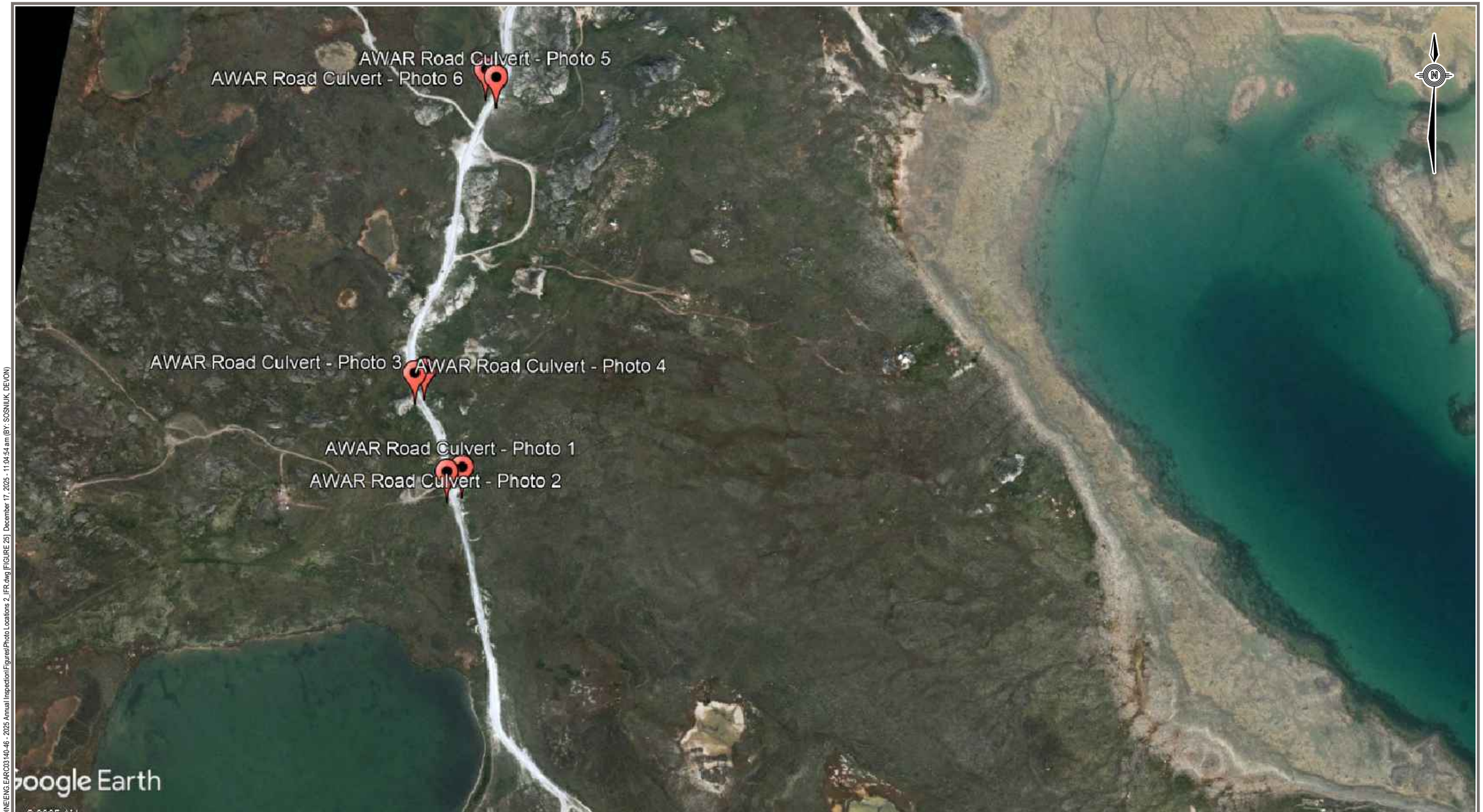
TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

ITIVIA BYPASS ROAD AND CULVERT PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0
OFFICE: EDM	DATE: December, 2025		

FIGURE 24



LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON AUGUST 10, 2024, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

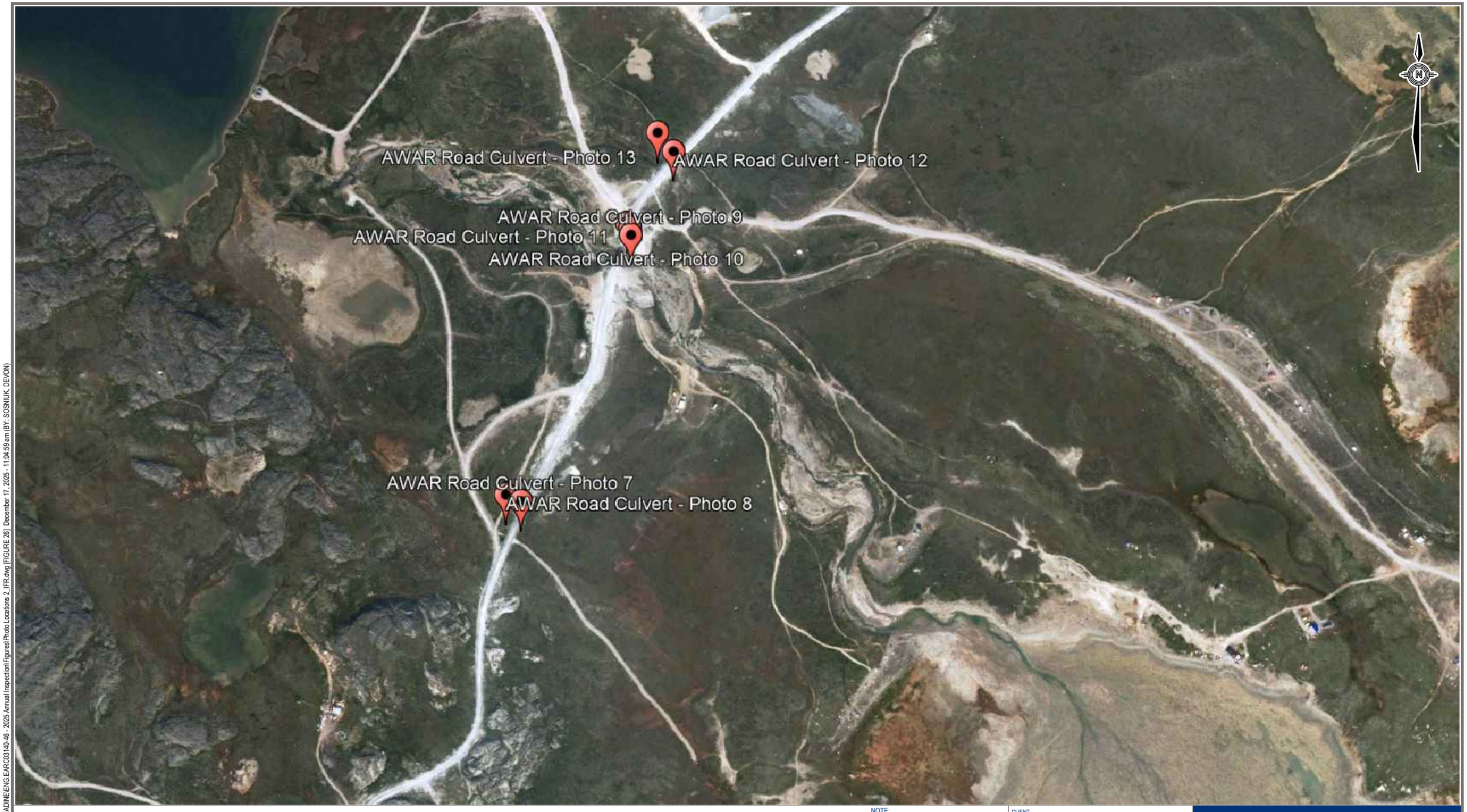
AWAR ROAD AND CULVERT PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	December, 2025

FIGURE 25

Q:\Edmonton\Engineering\E1411\Projects_MELIADINE\ENG.EARC03140-46 - 2025 Annual Inspection\Figures\Photo Locations 2_IFR.dwg [FIGURE 25] December 17, 2025 - 11:04:54 am (BY: SOSNIUK, DEVON)



LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON AUGUST 10, 2024, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

AWAR ROAD AND CULVERT PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	December, 2025

FIGURE 26

LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON AUGUST 10, 2024, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

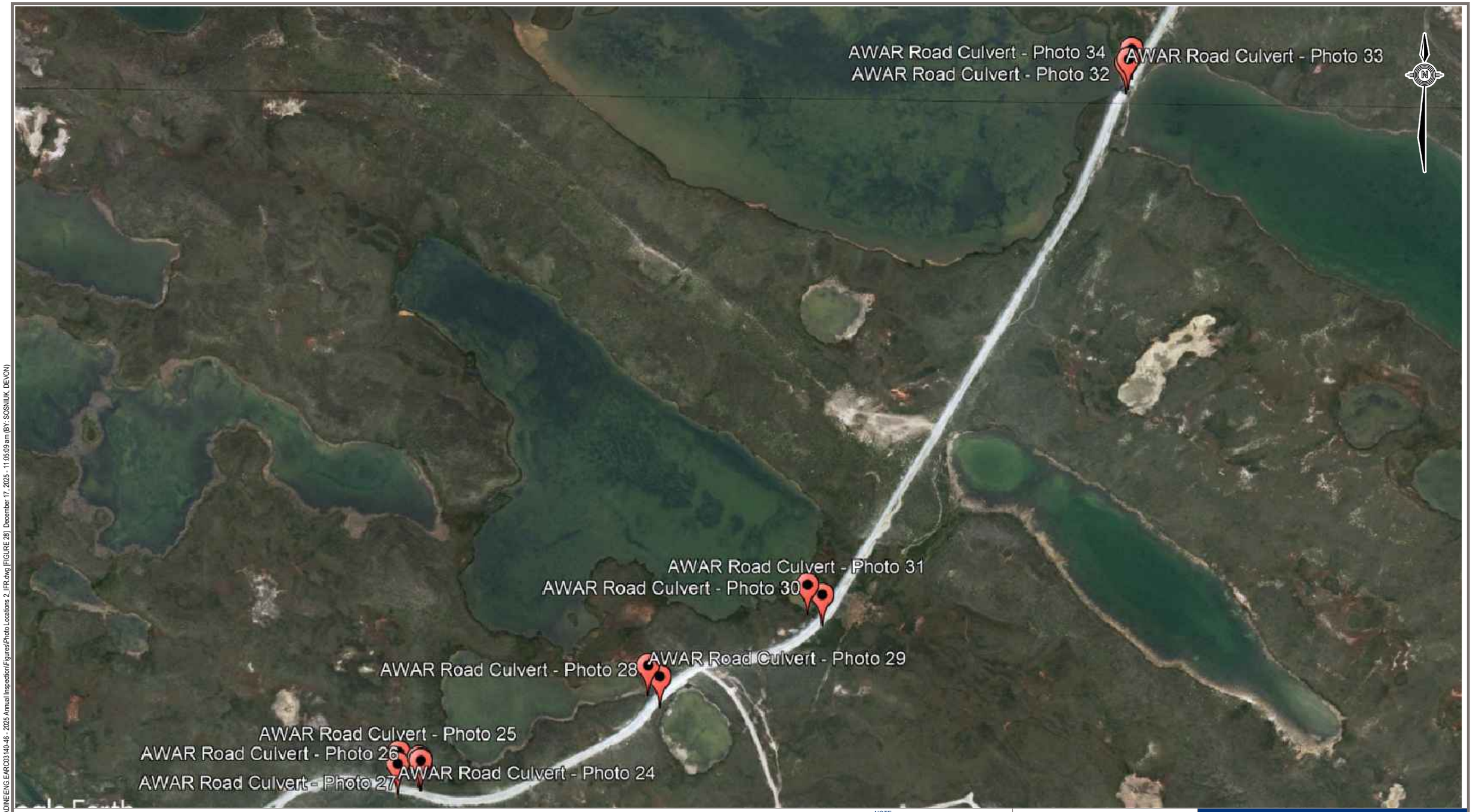
MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

AWAR ROAD AND CULVERT PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0
OFFICE: EDM	DATE: December, 2025		

FIGURE 27

Q:\Edmonton\Engineering\E141\Projects_MELIADINE\ENG.EARC03140-46 - 2025 Annual Inspection\Figures\Photo Locations 2_IFR.dwg [FIGURE 27] December 17, 2025 - 11:05:04 am (BY: SOSNIUK, DEVON)



LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON AUGUST 10, 2024, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

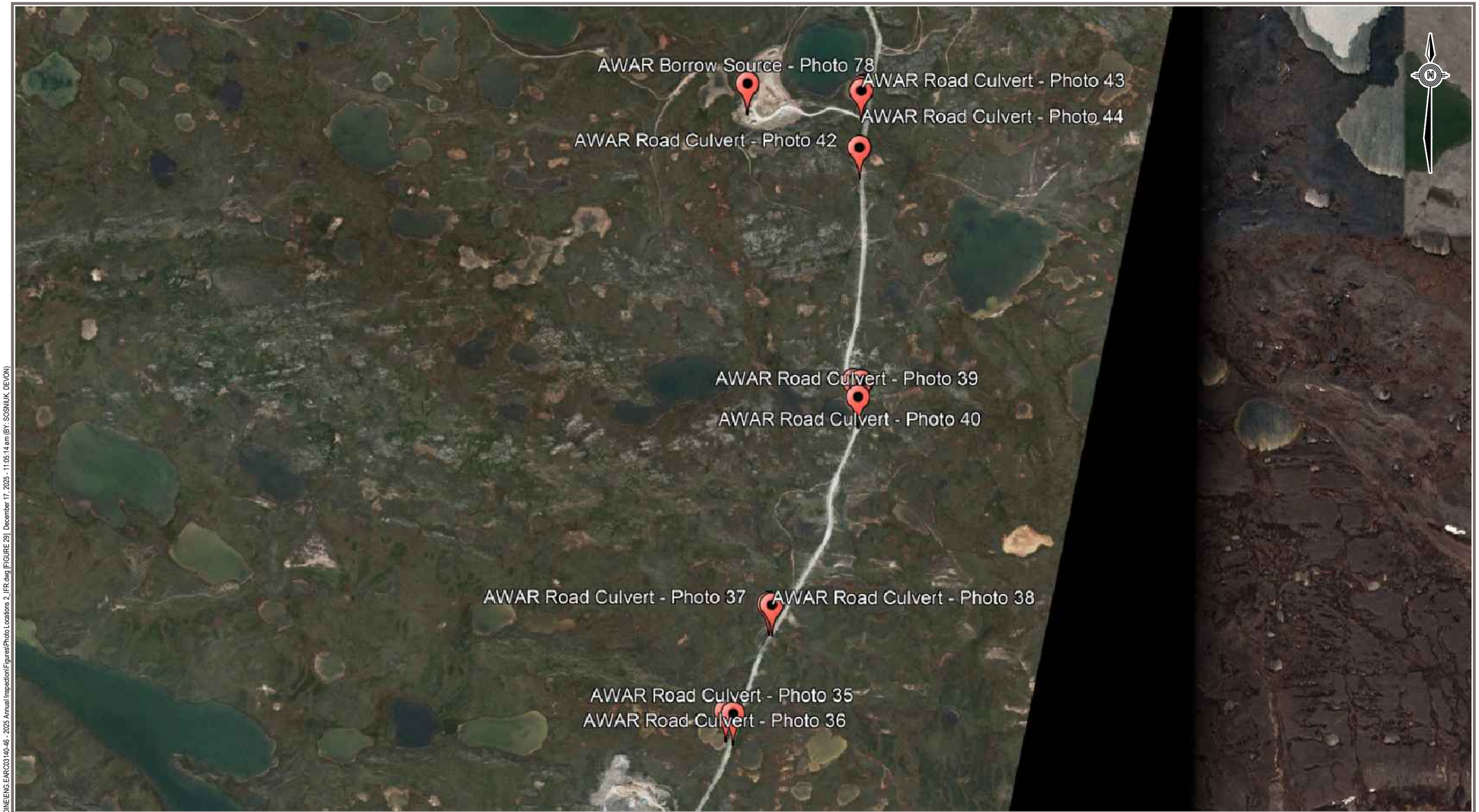
AWAR ROAD AND CULVERT PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	December, 2025

FIGURE 28

Q:\Edmonton\Engineering\E1441\Projects_MELIADINE\ENG.EARC03140-46 - 2025 Annual Inspection\Figures\Photo Locations 2_IFR.dwg [FIGURE 28] December 17, 2025 - 11:05:09 am (BY: SOSNIUK, DEVON)



LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON AUGUST 10, 2024, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

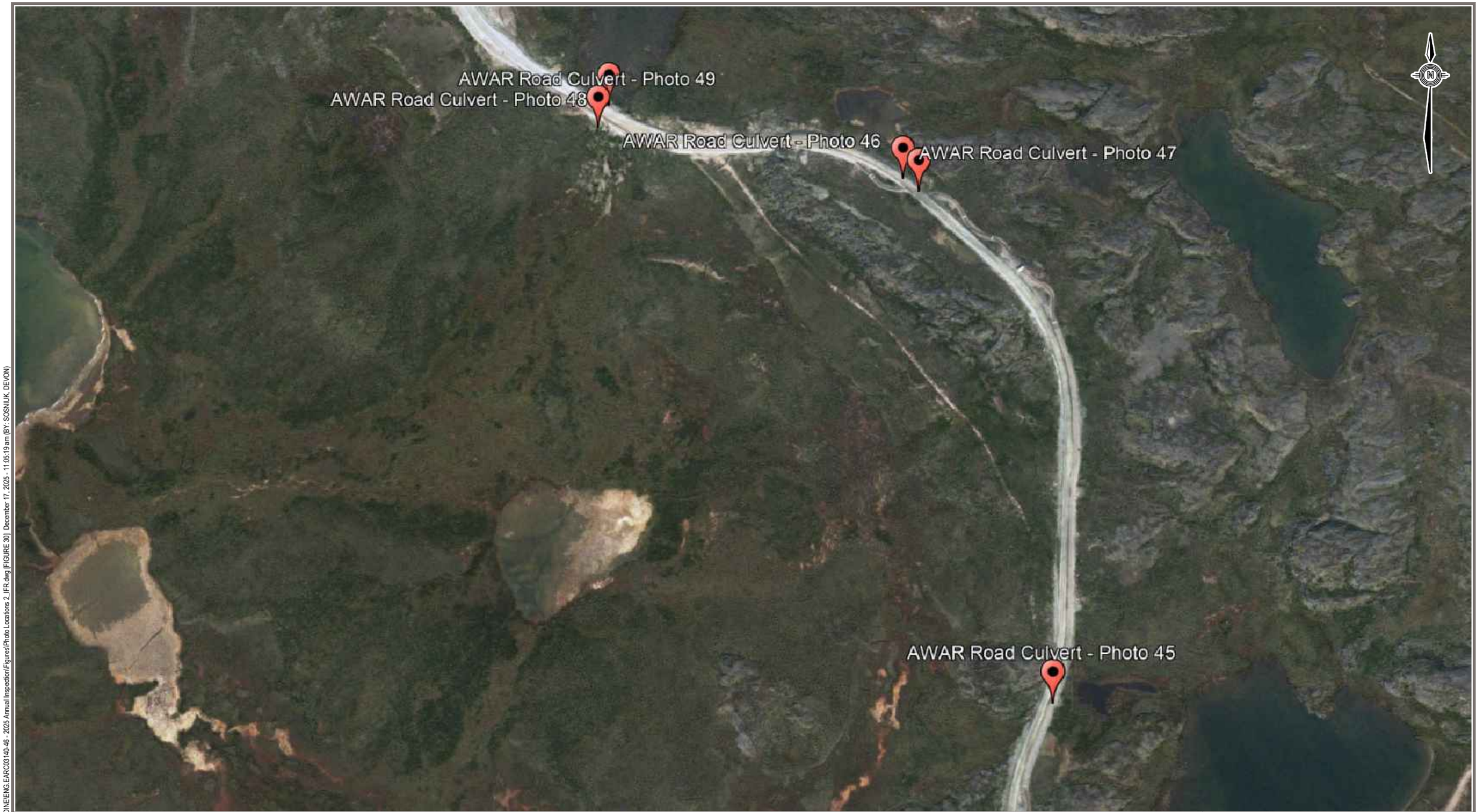
AWAR ROAD AND CULVERT PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-39	DS	HX	0

OFFICE	DATE
EDM	December, 2024

FIGURE 29

Q:\Edmonton\Engineering\E1411\Projects_MELIADINE\ENG.EARC03140-46 - 2025 Annual Inspection\Figures\Photo Locations 2_IFR.dwg [FIGURE 29] December 17, 2025 - 11:05:14 am (BY: SOSNIUK, DEVON)



LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON AUGUST 10, 2024, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT
AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

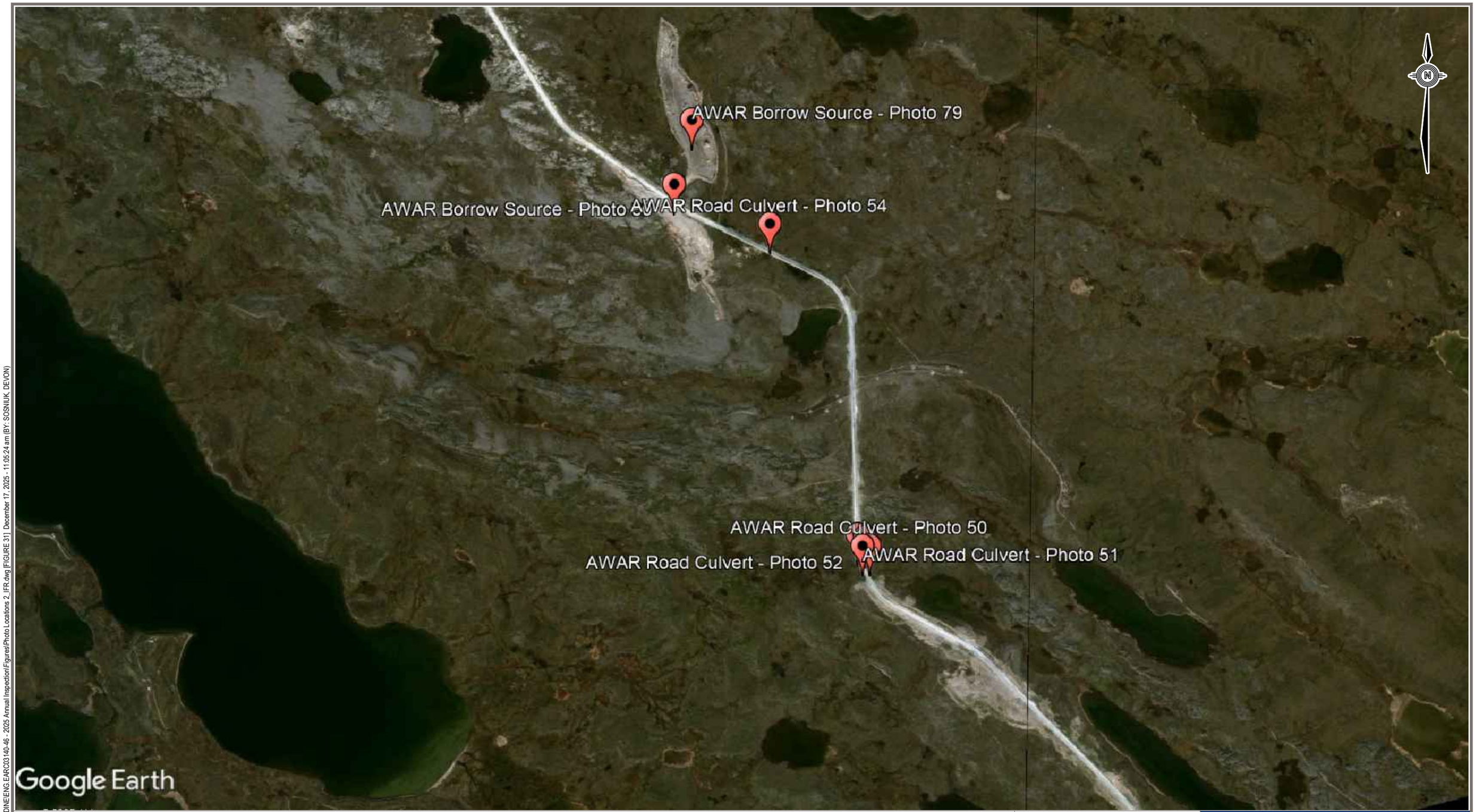
AWAR ROAD AND CULVERT PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	December, 2025

FIGURE 30

Q:\Edmonton\Engineering\E1411\Projects_MELIADINE\ENG.EARC03140-46 - 2025 Annual Inspection\Figures\Photo Locations 2_IFR.dwg [FIGURE 30] December 17, 2025 - 11:05:19 am (BY: SOSNIUK, DEVON)



LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON AUGUST 24, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

AWAR ROAD AND CULVERT PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	December, 2025

FIGURE 31

Q:\Edmonton\Engineering\E141\Projects_MELIADINE\ENG.EARC03140-46 - 2025 Annual Inspection\Figures\Photo Locations 2_IFR.dwg [FIGURE 31] December 17, 2025 - 11:05:24 am (BY: SOSNIUK, DEVON)

LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON AUGUST 24, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

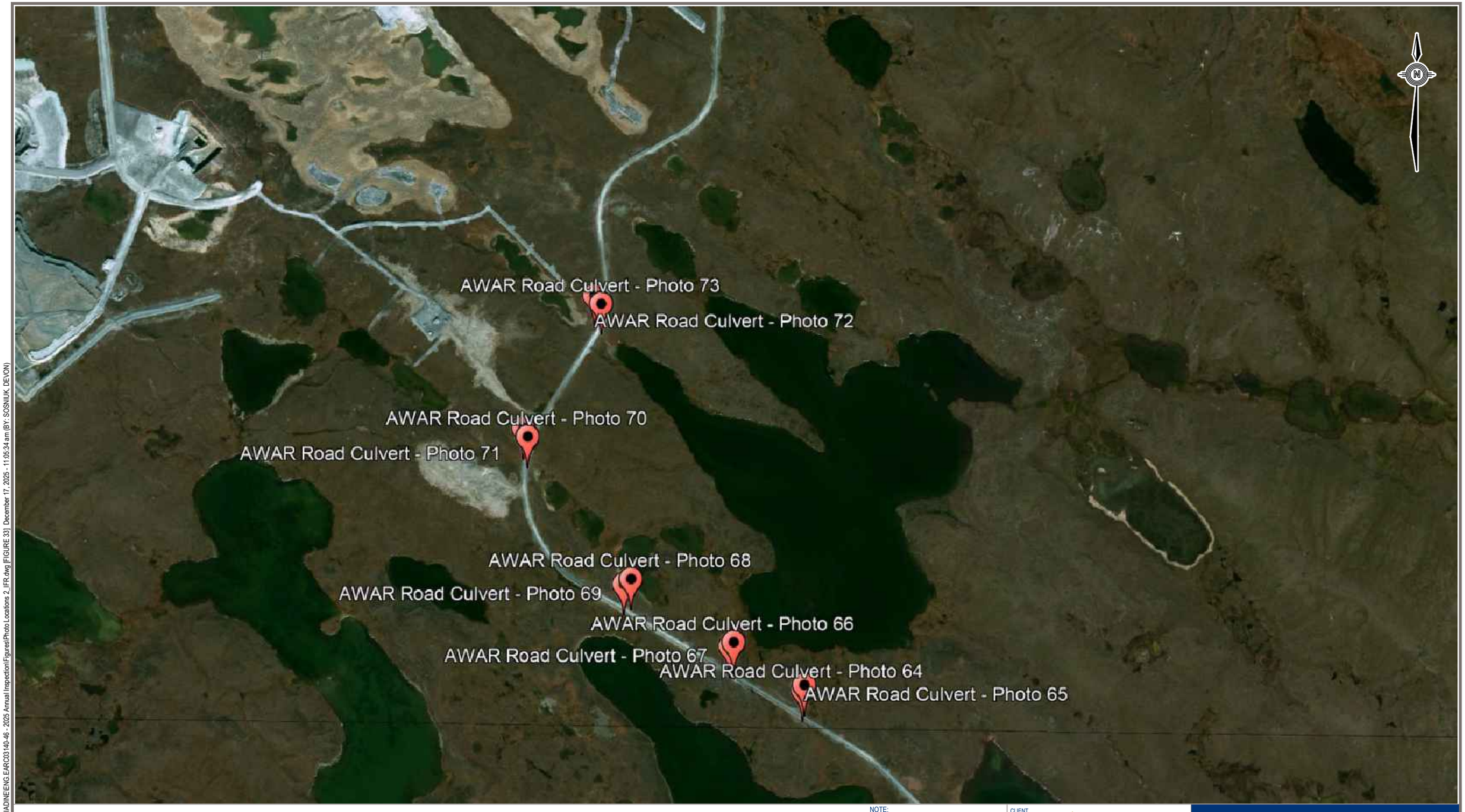
MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

AWAR ROAD AND CULVERT PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	December, 2025

FIGURE 32



LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGES TAKEN ON AUGUST 24 AND SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT
AGNICO EAGLE

TETRA TECH

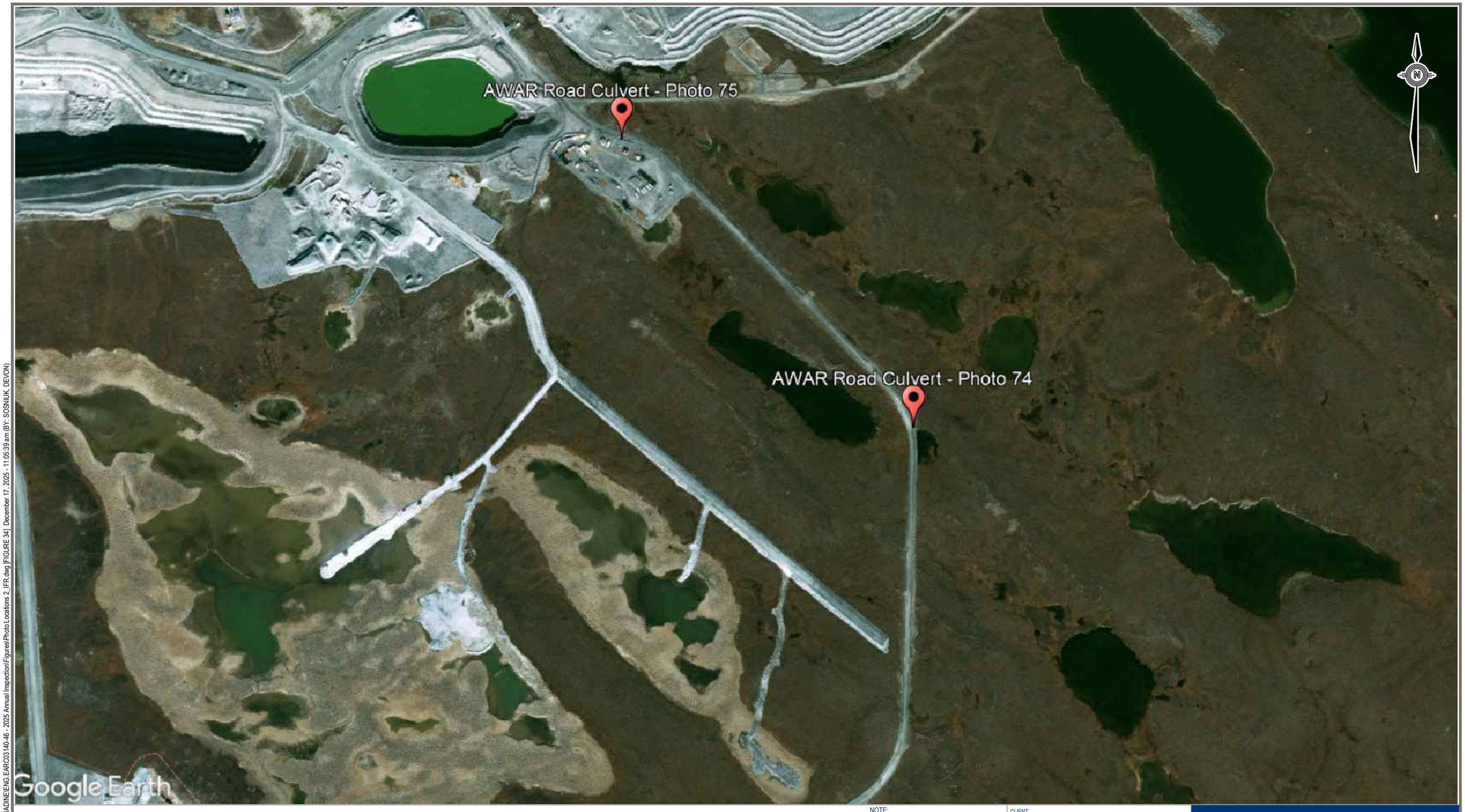
MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

AWAR ROAD AND CULVERT PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE: EDM
DATE: December, 2025

FIGURE 33



LEGEND:

- PHOTO LOCATION

NOTE:
SATELLITE IMAGE TAKEN ON SEPTEMBER 22, 2025, PROVIDED BY AGNICO EAGLE

ISSUED FOR USE

CLIENT

AGNICO EAGLE

TETRA TECH

MELIADINE GOLD MINE 2025 ANNUAL INSPECTION

AWAR ROAD AND CULVERT PHOTO LOCATIONS

PROJECT NO.	DWN	CKD	REV
ENG. EARC03140-46	DS	HX	0

OFFICE	DATE
EDM	December, 2025

FIGURE 34

APPENDIX A

TETRA TECH'S LIMITATIONS ON USE OF THIS DOCUMENT

LIMITATIONS ON USE OF THIS DOCUMENT

GEOTECHNICAL

1.1 USE OF DOCUMENT AND OWNERSHIP

This document pertains to a specific site, a specific development, and a specific scope of work. The document may include plans, drawings, profiles and other supporting documents that collectively constitute the document (the "Professional Document").

The Professional Document is intended for the sole use of TETRA TECH's Client (the "Client") as specifically identified in the TETRA TECH Services Agreement or other Contractual Agreement entered into with the Client (either of which is termed the "Contract" herein). TETRA TECH does not accept any responsibility for the accuracy of any of the data, analyses, recommendations or other contents of the Professional Document when it is used or relied upon by any party other than the Client, unless authorized in writing by TETRA TECH.

Any unauthorized use of the Professional Document is at the sole risk of the user. TETRA TECH accepts no responsibility whatsoever for any loss or damage where such loss or damage is alleged to be or, is in fact, caused by the unauthorized use of the Professional Document.

Where TETRA TECH has expressly authorized the use of the Professional Document by a third party (an "Authorized Party"), consideration for such authorization is the Authorized Party's acceptance of these Limitations on Use of this Document as well as any limitations on liability contained in the Contract with the Client (all of which is collectively termed the "Limitations on Liability"). The Authorized Party should carefully review both these Limitations on Use of this Document and the Contract prior to making any use of the Professional Document. Any use made of the Professional Document by an Authorized Party constitutes the Authorized Party's express acceptance of, and agreement to, the Limitations on Liability.

The Professional Document and any other form or type of data or documents generated by TETRA TECH during the performance of the work are TETRA TECH's professional work product and shall remain the copyright property of TETRA TECH.

The Professional Document is subject to copyright and shall not be reproduced either wholly or in part without the prior, written permission of TETRA TECH. Additional copies of the Document, if required, may be obtained upon request.

1.2 ALTERNATIVE DOCUMENT FORMAT

Where TETRA TECH submits electronic file and/or hard copy versions of the Professional Document or any drawings or other project-related documents and deliverables (collectively termed TETRA TECH's "Instruments of Professional Service"), only the signed and/or sealed versions shall be considered final. The original signed and/or sealed electronic file and/or hard copy version archived by TETRA TECH shall be deemed to be the original. TETRA TECH will archive a protected digital copy of the original signed and/or sealed version for a period of 10 years.

Both electronic file and/or hard copy versions of TETRA TECH's Instruments of Professional Service shall not, under any circumstances, be altered by any party except TETRA TECH. TETRA TECH's Instruments of Professional Service will be used only and exactly as submitted by TETRA TECH.

Electronic files submitted by TETRA TECH have been prepared and submitted using specific software and hardware systems. TETRA TECH makes no representation about the compatibility of these files with the Client's current or future software and hardware systems.

1.3 STANDARD OF CARE

Services performed by TETRA TECH for the Professional Document have been conducted in accordance with the Contract, in a manner consistent with the level of skill ordinarily exercised by members of the profession currently practicing under similar conditions in the jurisdiction in which the services are provided. Professional judgment has been applied in developing the conclusions and/or recommendations provided in this Professional Document. No warranty or guarantee, express or implied, is made concerning the test results, comments, recommendations, or any other portion of the Professional Document.

If any error or omission is detected by the Client or an Authorized Party, the error or omission must be immediately brought to the attention of TETRA TECH.

1.4 DISCLOSURE OF INFORMATION BY CLIENT

The Client acknowledges that it has fully cooperated with TETRA TECH with respect to the provision of all available information on the past, present, and proposed conditions on the site, including historical information respecting the use of the site. The Client further acknowledges that in order for TETRA TECH to properly provide the services contracted for in the Contract, TETRA TECH has relied upon the Client with respect to both the full disclosure and accuracy of any such information.

1.5 INFORMATION PROVIDED TO TETRA TECH BY OTHERS

During the performance of the work and the preparation of this Professional Document, TETRA TECH may have relied on information provided by persons other than the Client.

While TETRA TECH endeavours to verify the accuracy of such information, TETRA TECH accepts no responsibility for the accuracy or the reliability of such information even where inaccurate or unreliable information impacts any recommendations, design or other deliverables and causes the Client or an Authorized Party loss or damage.

1.6 GENERAL LIMITATIONS OF DOCUMENT

This Professional Document is based solely on the conditions presented and the data available to TETRA TECH at the time the data were collected in the field or gathered from available databases.

The Client, and any Authorized Party, acknowledges that the Professional Document is based on limited data and that the conclusions, opinions, and recommendations contained in the Professional Document are the result of the application of professional judgment to such limited data.

The Professional Document is not applicable to any other sites, nor should it be relied upon for types of development other than those to which it refers. Any variation from the site conditions present, or variation in assumed conditions which might form the basis of design or recommendations as outlined in this report, at or on the development proposed as of the date of the Professional Document requires a supplementary investigation and assessment.

TETRA TECH is neither qualified to, nor is it making, any recommendations with respect to the purchase, sale, investment or development of the property, the decisions on which are the sole responsibility of the Client.

1.7 ENVIRONMENTAL AND REGULATORY ISSUES

Unless stipulated in the report, TETRA TECH has not been retained to investigate, address or consider and has not investigated, addressed or considered any environmental or regulatory issues associated with development on the subject site.

1.8 NATURE AND EXACTNESS OF SOIL AND ROCK DESCRIPTIONS

Classification and identification of soils and rocks are based upon commonly accepted systems and methods employed in professional geotechnical practice. This report contains descriptions of the systems and methods used. Where deviations from the system or method prevail, they are specifically mentioned.

Classification and identification of geological units are judgmental in nature as to both type and condition. TETRA TECH does not warrant conditions represented herein as exact, but infers accuracy only to the extent that is common in practice.

Where subsurface conditions encountered during development are different from those described in this report, qualified geotechnical personnel should revisit the site and review recommendations in light of the actual conditions encountered.

1.9 LOGS OF TESTHOLES

The testhole logs are a compilation of conditions and classification of soils and rocks as obtained from field observations and laboratory testing of selected samples. Soil and rock zones have been interpreted. Change from one geological zone to the other, indicated on the logs as a distinct line, can be, in fact, transitional. The extent of transition is interpretive. Any circumstance which requires precise definition of soil or rock zone transition elevations may require further investigation and review.

1.10 STRATIGRAPHIC AND GEOLOGICAL INFORMATION

The stratigraphic and geological information indicated on drawings contained in this report are inferred from logs of test holes and/or soil/rock exposures. Stratigraphy is known only at the locations of the test hole or exposure. Actual geology and stratigraphy between test holes and/or exposures may vary from that shown on these drawings. Natural variations in geological conditions are inherent and are a function of the historic environment. TETRA TECH does not represent the conditions illustrated as exact but recognizes that variations will exist. Where knowledge of more precise locations of geological units is necessary, additional investigation and review may be necessary.

1.11 PROTECTION OF EXPOSED GROUND

Excavation and construction operations expose geological materials to climatic elements (freeze/thaw, wet/dry) and/or mechanical disturbance which can cause severe deterioration. Unless otherwise specifically indicated in this report, the walls and floors of excavations must be protected from the elements, particularly moisture, desiccation, frost action and construction traffic.

1.12 SUPPORT OF ADJACENT GROUND AND STRUCTURES

Unless otherwise specifically advised, support of ground and structures adjacent to the anticipated construction and preservation of adjacent ground and structures from the adverse impact of construction activity is required.

1.13 INFLUENCE OF CONSTRUCTION ACTIVITY

There is a direct correlation between construction activity and structural performance of adjacent buildings and other installations. The influence of all anticipated construction activities should be considered by the contractor, owner, architect and prime engineer in consultation with a geotechnical engineer when the final design and construction techniques are known.

1.14 OBSERVATIONS DURING CONSTRUCTION

Because of the nature of geological deposits, the judgmental nature of geotechnical engineering, as well as the potential of adverse circumstances arising from construction activity, observations during site preparation, excavation and construction should be carried out by a geotechnical engineer. These observations may then serve as the basis for confirmation and/or alteration of geotechnical recommendations or design guidelines presented herein.

1.15 DRAINAGE SYSTEMS

Where temporary or permanent drainage systems are installed within or around a structure, the systems which will be installed must protect the structure from loss of ground due to internal erosion and must be designed so as to assure continued performance of the drains. Specific design detail of such systems should be developed or reviewed by the geotechnical engineer. Unless otherwise specified, it is a condition of this report that effective temporary and permanent drainage systems are required and that they must be considered in relation to project purpose and function.

1.16 BEARING CAPACITY

Design bearing capacities, loads and allowable stresses quoted in this report relate to a specific soil or rock type and condition. Construction activity and environmental circumstances can materially change the condition of soil or rock. The elevation at which a soil or rock type occurs is variable. It is a requirement of this report that structural elements be founded in and/or upon geological materials of the type and in the condition assumed. Sufficient observations should be made by qualified geotechnical personnel during construction to assure that the soil and/or rock conditions assumed in this report in fact exist at the site.

1.17 SAMPLES

TETRA TECH will retain all soil and rock samples for 30 days after this report is issued. Further storage or transfer of samples can be made at the Client's expense upon written request, otherwise samples will be discarded.

APPENDIX B

POND CP1 AND DIKE D-CP1